

# APPENDIX B – SPACING AND CAPACITY EVALUATIONS FOR DIFFERENT AHS CONCEPTS

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## B. ABSTRACT

In Automated Highway Systems (AHS), vehicles will be able to follow each other automatically by using their own sensing and control systems, effectively reducing the role of the human driver in the operation of the vehicle. Such systems are therefore capable of reducing one source of error, human error, that diminishes the potential capacity of the highways and in the worst case becomes the cause of accidents. The inter-vehicle separation during vehicle following is one of the most critical parameters of the AHS system, as it affects both safety and highway capacity. To achieve the goal of improved highway capacity, the inter-vehicle separation should be as small as possible. On the other hand, to achieve the goal of improved safety and elimination of rear end collisions, the inter-vehicle separation should be large enough that even under a worst case stopping scenario, no vehicle collisions will take place. These two requirements demand diametrically opposing solutions and they have to be traded off. Since safety cannot be compromised for the sake of capacity, it becomes a serious constraint in most AHS design decisions. The trade-off between capacity and safety gives rise to a variety of different AHS concepts and architectures.

In this study we consider a family of six AHS operational concepts. For each concept we calculate the minimum inter-vehicle spacing that could be used for collision-free vehicle following, under different road conditions. The minimum spacing in turn, is used to calculate the maximum possible capacity that could be achieved for each operational concept.

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## B.1- INTRODUCTION

Urban highways in many major cities are congested and need additional capacity. Historically, capacity has been added by building additional lanes and new highways. Scarcity of land and escalating construction costs make it increasingly difficult to add capacity this way. One possible way to improve capacity is to use current highways more efficiently. The concept of Automated Highway Systems (AHS) was introduced to improve the capacity of the current transportation systems by using automation and intelligence.

Highway capacity depends on two variables: The velocity of the vehicles and the distance between them. Clearly, the higher the velocity of the vehicles, the higher the number of vehicles per lane per hour will be. But the vehicles need to maintain a certain amount of "safety distance" between them, to accommodate for the case that the flow of vehicles has to be slowed down or stopped, by applying the brakes. The moment that each vehicle starts applying its brakes typically involves a couple of seconds of delay in relation to the onset of braking of the vehicle in front, due to the fact that the human drivers need some time to process the information they perceive<sup>[22]</sup>, plus an additional time delay to react and a delay for the mechanical and hydraulic systems of the vehicle to respond. During this time, the vehicle continues moving forward at practically the same speed and if there is not sufficient space between the leading and the following vehicle at the moment the leading vehicle applies the brakes and begins to decelerate, a collision would be inevitable. Even if the follower begins to apply its brakes at exactly the same time as the leader, the deceleration of the leading and the following vehicle may not match<sup>[9,10]</sup> and this generates the need for additional inter-vehicle distance during the cruising stage in order to accommodate for the difference in braking performance.

Heavy vehicles travel a significantly longer distance from the moment they apply their brakes until they come to a complete stop. This has to be accommodated for by allowing a significantly larger inter-vehicle

spacing. On the other hand, when a light vehicle follows a heavy vehicle, the braking distance is not the limiting factor because typically the light vehicle will be able to come to a stop in a much shorter time and distance. In this case, the limiting factors are the initial conditions and the total delay between the time that the leader starts decelerating and the time that the follower starts decelerating at the maximum possible deceleration.

The delay in detecting and in reacting to the leading vehicle's deceleration can be reduced significantly, by taking the human driver out of the "control loop"<sup>[1,12,13,16]</sup>. With advances in technology and vehicle electronics, systems that were previously considered impossible to implement or too costly are becoming feasible and available. One such system is a functional extension of the classic cruise control<sup>[12]</sup>. The cruise control which is widely available on luxury cars today, is a controller that controls a throttle actuator in order to maintain constant vehicle speed. The next step in functionality, is a controller that uses a sensor to measure the relative distance and the relative speed to any vehicle ahead and controls a throttle and a brake actuator in order to follow at the same speed and maintain a fixed relative distance<sup>[12,14,15]</sup>. Such vehicles can follow each other in the same lane automatically by relying on their own sensors and controls. Vehicles that rely on their own sensors, controls and intelligence to operate in a highway environment are referred to as autonomous vehicles.

Advances in communications made it possible for vehicles to communicate with each other exchanging information about braking intentions and capabilities, acceleration, lane changing etc. The infrastructure may also support vehicle following and maneuvers by providing desired speed and spacing commands in addition to traveler information. This distribution of intelligence gives rise to the operating concept referred to as infrastructure supported free agent.

When the infrastructure becomes actively involved by sending braking commands for

emergency stops and lane changing maneuvers, we have an operating concept referred to as infrastructure managed free agent.

Another concept is to organize free agent vehicles in platoons of a certain size where the intra-platoon spacing is very small and the inter-platoon spacing could be larger for safety purposes. In this case each platoon appears to the infrastructure as a single unit and therefore can be managed more efficiently. Each platoon is now responsible for the control of its vehicles so that a collision free environment is guaranteed.

In another concept, a high level of synchronization is introduced where each vehicle is allocated a slot in time and space. The infrastructure manages the slot distribution by issuing the appropriate commands for each vehicle.

The degree of infrastructure involvement and distribution of intelligence lead to different operational concepts and architectures for AHS. The purpose of this paper is to study the Minimum Safety Spacing (MSS) for a number of different AHS concepts and architectures and to obtain capacity estimates.

The paper is organized as follows: Section 2 presents the fundamental equations used in computing the MSS. Section 3 describes the candidate Vehicle Following Concept options. Section 4 presents the Spacing and Capacity calculations for each concept. Section 5 contains some discussion and further explanation of the results.

## B.2 MINIMUM SAFETY SPACING

Inter-vehicle spacing during vehicle following is a very critical parameter of highway traffic. Insufficient spacing is usually the cause of rear-end collisions. In principle, the possibility of having a rear-end collision can be reduced by increasing the inter-vehicle spacing. However, the spacing that guarantees collision-free vehicle following can be characterized only when the braking scenario is known and well defined.

A braking scenario, which describes exactly how the vehicles brake, is usually specified by the deceleration profiles of the vehicles as a function of time. For each scenario there is a minimum spacing which must be maintained during steady state traffic flow, if collision-free vehicle following must be guaranteed. In this section we develop the basic equations that can be used to calculate the minimum spacing for collision free vehicle following, given the deceleration response information for both the leading and the following vehicle.

### B.2.1 Safe Intervehicle Spacing Analysis

Consider two vehicles following each other, as shown in Figure B.2.1-1. Assume that at  $t=0$  the leading vehicle begins to brake according to the deceleration profile defined by  $a_l(t)$  and the following vehicle brakes according to the deceleration profile defined by  $a_f(t)$ . Assume that  $L_l$  and  $L_f$  are the lengths of the leading and following vehicles respectively. At  $t=0$  the leading vehicle has a velocity  $V_l(0)=V_{l0}$  and a position  $S_l(0)=S_{l0}$  and the following vehicle has a velocity  $V_f(0)=V_{f0}$  and a position  $S_f(0)=S_{f0}$ . If the spacing between the two vehicles at  $t=0$ ,  $S_r(0) = S_{l0} - S_{f0} - L_l$  is large enough, then there would be no collision during braking maneuvers.

For a given braking scenario we would like to calculate the minimum value of the initial intervehicle spacing  $S_r(0)$  for which there will be no collision. We refer to this value as the Minimum Safety Spacing (MSS).

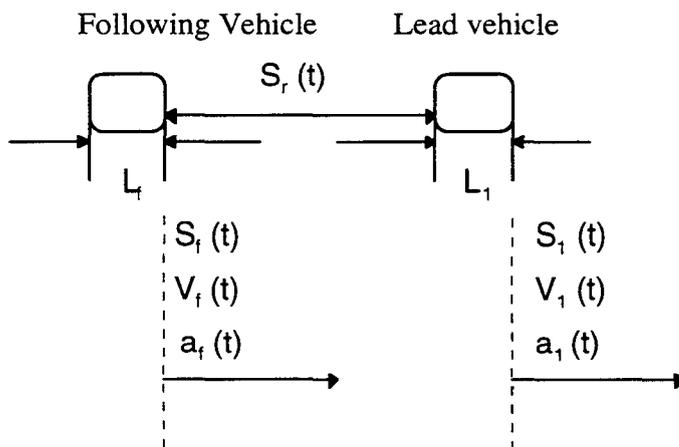


Figure B.2.1-1. Vehicle Following

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The spacing between the two vehicles measured from the front of the following vehicle to the rear of the lead vehicle is given by

Eq. 1

$$S_r(t) = S_l(t) - L_l - S_f(t)$$

where

Eq. 2

$$S_l(t) = S_l(0) + \int_0^t V_l(\tau) d(\tau)$$

Eq. 3

$$S_f(t) = S_f(0) + \int_0^t V_f(\tau) d\tau$$

and

Eq. 4

$$V_l(t) = V_l(0) + \int_0^t a_l(\tau) d\tau$$

Eq. 5

$$V_f(t) = V_f(0) + \int_0^t a_f(\tau) d\tau$$

If the decelerations  $a_l(t)$  and  $a_f(t)$  and initial positions and velocities are specified, the MSS can be calculated as follows:

Assume that the two vehicles travel in the same direction but in two separate lanes. The position of the vehicles at time  $t = 0$  is shown in Figure B.2.1-2.

Let  $t_s$  be the stopping time of the following vehicle. Then

Eq. 6

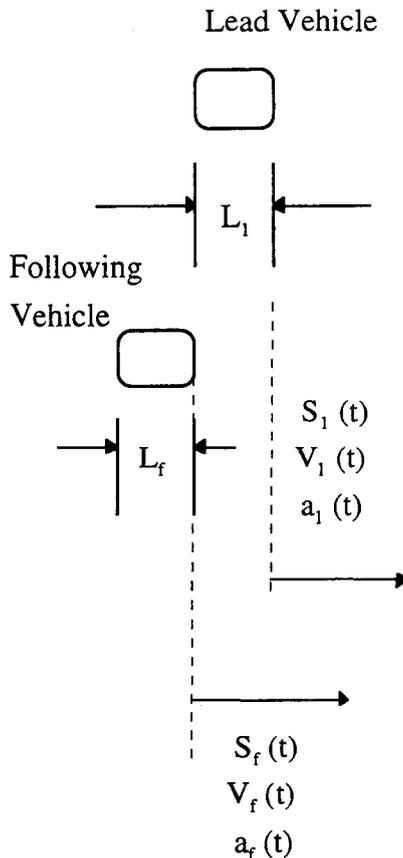
$$V_f(0) + \int_0^{t_s} a_f(\tau) d(\tau) = 0$$

Eq. 7

$$S_f(t) = S_f(0) + \int_0^t V_f(\tau) d(\tau), \forall t \leq t_s$$

and Eq. 8

$$S_f(t) = S_f(t_s), \forall t > t_s$$



**Figure B.2.1-2. Hypothetical Vehicle Motion**

The position of the leading vehicle at each time  $t$  is given by

Eq. 9

$$S_l(t) = S_l(0) + \int_0^t V_l(\tau) d(\tau), \forall t \leq t_s$$

The relative spacing at each time  $t$  is given by

Eq. 10

$$S_r(t) = S_l(t) - L_l - S_f(t)$$

If both the leading and following vehicle are in the same lane, then  $S_r(t) > 0$  for all  $t \in (0, t_s]$  will imply no collision, whereas  $S_r(t) < 0$  at some  $t = t_c \in (0, t_s]$  will imply collision.

The MSS value denoted by  $S_{min}$  is given as  $S_{min} = - \min S_r(t), t \in (0, t_s]$

In other words  $S_{min}$  is equal to the maximum distance by which the following vehicle would overtake the leading vehicle at any time  $t$  in the interval  $[0, t_s]$  in the scenario shown in Figure B.2.1-2.

Based on the above analysis, we adopt a numerical method to calculate  $S_{min}$ . Assume that the following vehicle brakes and it does so by following the given deceleration profile, and comes to a full stop at  $t=t_s$ . We divide the interval  $[0, t_s]$  into small time steps and consider the time instants  $t = 0, T_s, 2T_s, \dots, kT_s$  where  $T_s$  is the length of the time step and  $k$  is an integer with the property  $kT_s \leq (k+1)T_s$ . The method of calculation of  $S_{min}$  is shown in the flowchart of Figure B.2.1-3.

## B.3 VEHICLE FOLLOWING CONCEPTS

### B.3.1 Motivation

With advances in technology and in particular in vehicle electronics, systems that were previously considered impossible to implement or too costly are becoming feasible and available. One such system is a functional extension of the classic cruise control. It consists of a controller that uses a sensor to measure the relative distance and the relative speed to any vehicle ahead and controls a throttle and a brake actuator in order to follow at the same speed and maintain a desired relative distance. The relative distance may be characterized in terms of a constant length or it may be a function of the speed. If the majority of vehicles have such a controller on board, we can have an environment where vehicles follow each other automatically, in the same highway lane, without any other kind of interaction such as communication between them. The highway may provide a level of support to the vehicles by transmitting information about road conditions, congestion, routing suggestions and possibly recommended speeds. If the vehicles do not communicate and do not require any infrastructure support they are said to operate autonomously. A system like that, may provide a capacity increase by

smoothing out traffic flow and eliminating the mistake that human drivers tend to do, that is to follow at short and unsafe distances and then overcorrecting by slowing down too much when a vehicle ahead starts to decelerate.

A further functionality enhancement comes by allowing the vehicles to communicate and notify each other about their braking intentions. Also the infrastructure may become involved in setting the desired velocity for each section of the highway communicating to vehicles about the need for emergency braking and coordinating the flow of the traffic. Such systems may achieve significant improvements in flow rates and capacity increases of the existing highways. In this section we describe a number of operating AHS concepts for automatic vehicle following.

### B.3.2 Autonomous Vehicles

The simplest architecture is when the vehicles operate independently i.e., autonomously, using their own sensors. Each vehicle senses its environment, including lane position, adjacent vehicles and obstacles. The infrastructure may provide basic traveler information services, i.e., road conditions and routing information. The infrastructure may also provide some means to assist the vehicle in sensing its lane position. Many different systems have been proposed to help the vehicle sense its position, such as implanted magnetic nails, magnetic stripes, radar reflective stripes, RF cables, or GPS satellites<sup>[23]</sup>.

In an autonomous environment, the vehicle does not rely on communication with other vehicles or the infrastructure in order to make vehicle following decisions. Each autonomous vehicle maintains a safe distance from the vehicle it is following or if a vehicle is not present within the sensing distance it travels at a constant speed in accordance with the posted speed limits and regional safety regulations and of course road conditions. In other words, if there is no vehicle ahead within the maximum safety distance, the vehicle travels at the speed limit or at a lower speed depending on the conditions.

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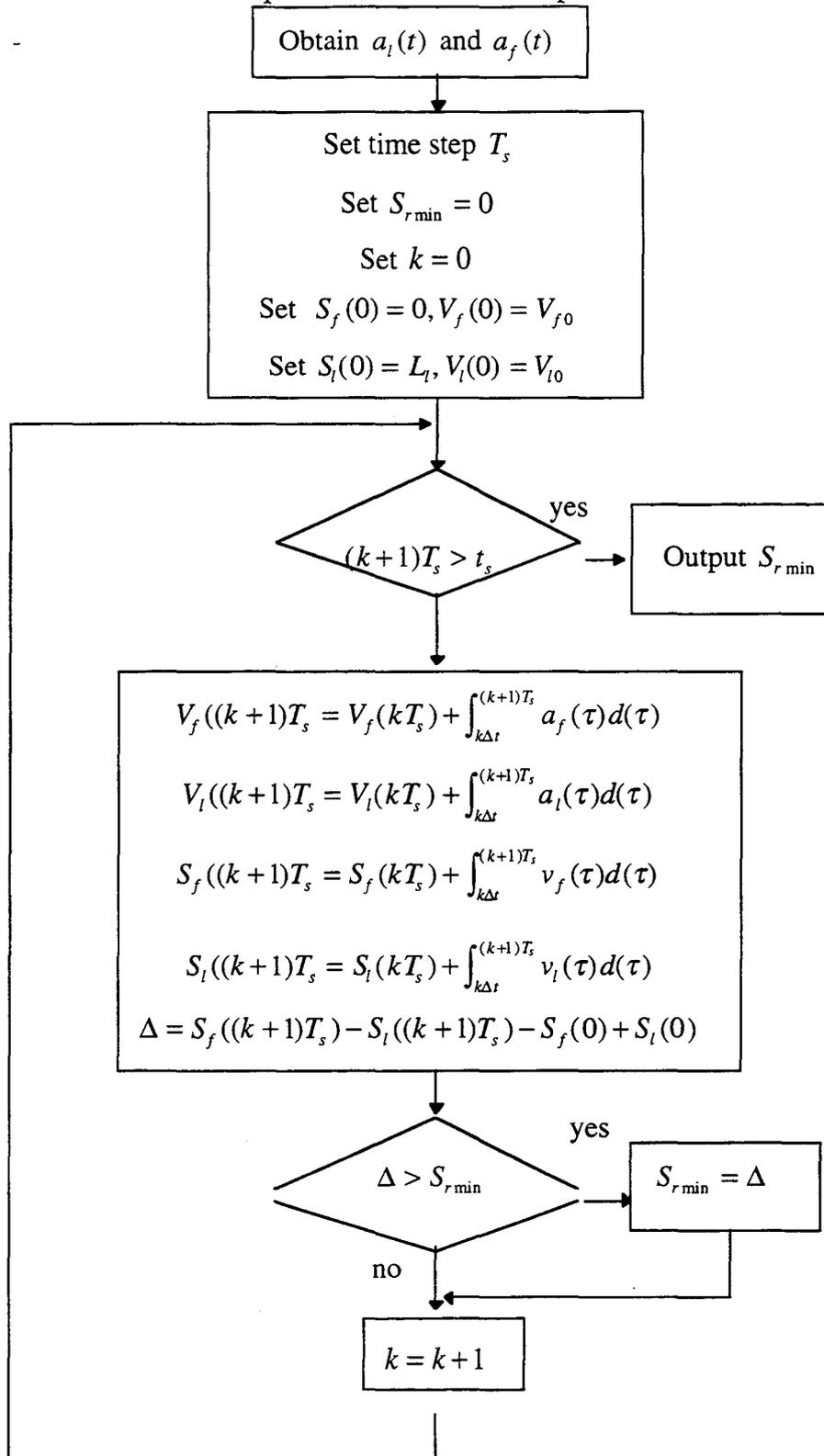


Figure B.2.1-3. Flowchart for MSS Calculation

Since there is no communication between vehicles for separation control, each vehicle senses the relative spacing and speed to the vehicle ahead and decides and selects a headway based on its own braking capabilities. The technology that allows the vehicle to sense the relative position and speed to the vehicle ahead can also be adapted to allow the vehicle to estimate the size and indirectly the vehicle class and braking capabilities of the vehicle ahead. The availability of this technology is not required but it will affect the capacity when there is mixing of vehicle classes, i.e., mixing of autonomous passenger vehicles, buses and heavy trucks as we will show in section 4.

### **B.3.3 Free Agent Vehicles - Infrastructure Supported**

A vehicle is considered a Free Agent if it has the capability to operate autonomously but it is also able to receive communications from other vehicles and from the infrastructure. This implies that the infrastructure may get involved in a supporting role, by issuing warnings and recommendations for desired speed and headways but the infrastructure will not have the authority to issue direct control commands. Therefore this concept has been named “Infrastructure Supported”. The fundamental difference between this concept and that described in subsection B.3.2 is that there is vehicle to vehicle and vehicle to infrastructure communication. Each vehicle communicates to the vehicle behind its braking capabilities and its braking intentions. This allows the vehicle behind to choose its headway. For example a shorter headway can be selected by a passenger vehicle if the vehicle ahead is a heavy truck or a bus. A larger headway must be selected by a heavy vehicle if the vehicle ahead is a passenger vehicle. A free agent vehicle uses its own sensors to sense its position and environment, including lane position, adjacent vehicles and obstacles.

The MSS between vehicles is expected to be smaller than that on conventional highways because of the intelligent longitudinal control system and vehicle to vehicle and

infrastructure to vehicle communications. Each vehicle senses the relative spacing and speed to the vehicle ahead and decides and selects a headway based on its own braking capability, the braking capability of the vehicle ahead and the road surface conditions which are either sensed by the vehicle or are broadcasted from the infrastructure. When a vehicle starts to brake, it notifies the vehicle behind about the magnitude of its braking force. Even if we assumed a relatively primitive form of communication between vehicles like a line of sight communication that transmits the applied braking force, we can achieve better separation control as we eliminate the delay in deciding if the vehicle ahead is performing emergency braking.

### **B.3.4 Free Agent Vehicles - Infrastructure Managed**

The Free Agent vehicles with Infrastructure Management is based on the assumption that the traffic is composed of vehicles acting as free agents while the infrastructure assumes a more active and more complex role in the coordination of the traffic flow and control of vehicles. Each vehicle is able to operate autonomously and uses its own sensors to sense its position and environment, including lane position, adjacent vehicles and obstacles. The difference in this centrally managed architecture is that the infrastructure has the ability to send commands to individual vehicles.

This is envisioned to be a “request-response” type architecture, in which individual vehicles ask permission from the infrastructure to perform certain activities and the infrastructure responds by sending commands back to the requesting vehicle and to other vehicles in the neighborhood.

It is expected and assumed that the infrastructure is able to detect emergency situations and whenever it detects such emergency, the infrastructure will have the responsibility to send an emergency braking command to all vehicles affected. This concept minimizes the delay in performing emergency braking. This allows for some further reduction of the minimum headway, compared to the other architectures

presented so far. On the other side, the accurate timing of the emergency and stopping commands for each vehicle that must be issued by the infrastructure, requires accurate tracking of individual vehicles as well as extensive and frequent communications between individual vehicles and the infrastructure.

### **B.3.5 Platooning Without Coordinated Braking**

This concept represents the possibility that the safest and possibly most cost-effective way of achieving maximum throughput is by making platoons of vehicles the basic controlling unit. This will boost road capacity by expanding on the concept of infrastructure managed control<sup>[17,18,19]</sup>.

Platoons are clusters of vehicles with short spacing between individual vehicles in each group and longer spacing between platoons. The characterizing differentiation is that the platoon is to be treated by the infrastructure as an “entity” thereby minimizing some of the need for communicating with and coordinating individual vehicles. The infrastructure does not attempt to control any individual vehicle under normal circumstances, keeping the cost and necessary bandwidth low. The infrastructure is expected to be an intelligent agent which monitors and coordinates the operation of the platoons.

Tight coordination is required within the platoon in order to maintain a close spacing and this requires that the vehicles must be communicating with each other, constantly. The significantly longer inter-platoon spacing is required to guarantee no inter-platoon collisions.

Each vehicle is expected to be equipped with the sensors and intelligence to maintain its lane position, sense its immediate surroundings, and perform the functions of merging into and splitting off a platoon. It is not expected to accomplish lane changes, or merging and splitting without the infrastructure’s or the platoon entity’s help.

The main mode of operation of the infrastructure would be of a request-response type. Each vehicle’s requests are

processed and appropriate commands are sent to the appropriate vehicles/platoons to respond to that request. The infrastructure takes a more pro-active role in monitoring traffic flow, broadcasting traffic flow messages, advising lane changes to individual vehicles and platoons in addition to the usual information provider functions.

Once a vehicle has merged into a platoon, the headway maintenance controller must take into account the braking capabilities of each vehicle in the platoon in order to set an optimal separation distance that minimizes the possibility of collision.

Mixing of vehicle classes, although an implicit feature of the present highway system, creates a major complication because of the dissimilar braking characteristics of each vehicle class. Therefore it makes sense to form platoons of vehicles belonging to the same class, exclusively.

### **B.3.6 Platooning with Coordinated Braking**

The Platooning architecture with Coordinated Braking is based on the concept of maximizing capacity by carefully coordinating the timing and degree of braking among the vehicles participating in a platoon entity. This allows the minimization of the spacing between vehicles without compromising safety.

The distinguishing feature of this concept is the minimization of intra-platoon spacing and the promise of higher capacity. Platooning, complete vehicle automation, global traffic flow management and controlled routing of different vehicle classes are important factors in achieving that goal. However, infrastructure investment and the complexity of the communication system that is required will be an important cost factor.

The intelligence will reside both on the vehicles and on the infrastructure. The vehicle uses the on-board intelligent control systems mainly for longitudinal control and platooning functions and also for lateral control.

The bulk of the communication will probably take place between vehicles. Vehicles that are at some distance apart are not likely to have a need to communicate as their dynamics and trajectories do not affect each other. At the same time it is desirable to minimize the transmitting power and range of vehicle to vehicle communication to minimize interference to other vehicles and to allow for efficient spectrum reuse.

### **B.3.7 Infrastructure Managed Slotting**

Under the Infrastructure Managed Slotting concept, an infrastructure based control system creates and maintains vehicle “slots” in space and time. Slots can be thought of as moving roadway segments, each of which holds at most one vehicle at any time. The vehicles are identified and managed only by association with these slots. For simplicity in management i.e., to achieve slots of uniform length, vehicles that need more space may be assigned multiple slots. Heavy loaded light trucks may be assigned two slots, unloaded semis may be assigned three slots, loaded heavy trucks may be assigned four slots etc.

The basic slotting concept is that the slots should be of fixed length. The virtual leading edge of each slot can be thought of as a moving point that the vehicle assigned to the slot has to follow. Thus the controller on the vehicle is assigned to follow this virtual moving point, not another vehicle. In essence this relieves the requirement of using headway sensors on the vehicle and of sensing the relative distance and speed to any other vehicle. Under no circumstances is a vehicle allowed to violate the edges of its assigned slot.

The distinguishing feature of this concept is that the sensing requirements are theoretically simplified. At least, the vehicle does not need to sense the relative position and speed of other vehicles. Yet the vehicle must be able to sense its position relative to the edge of the slot and the virtual point it tries to follow. A global and accurate longitudinal position sensing system is required.

In terms of separation policy, the slotting method is bounded by the limitations of the inherently “synchronous” architecture. This means that the size of each slot must be sufficient such that the spacing between individual vehicles occupying a single slot is sufficient to avoid collisions under the worst case scenario. Thus the weakest link in the chain is the vehicle with the worst braking performance that the system tries to accommodate in a single slot. Once the spacing is set to accommodate such a vehicle, every other vehicle which has better braking performance will not be able to utilize this capability to shorten the spacing to the vehicle in front. There will be “dead space” in between them. Similarly, a vehicle that does not meet the minimum braking requirement to occupy a single slot will be assigned two (or more) consecutive slots, with the resulting inefficiency of wasting even more space than is really needed.

By comparison, an architecture where each vehicle optimizes the headway between itself and the vehicle in front based only on the braking capabilities of the two vehicles involved is inherently an “asynchronous” architecture, which results in true minimization of the unused space between vehicles.

The relative merits of a “synchronous” versus an “asynchronous” architecture have been intriguing the designers of computers and communications systems ever since digital systems became a reality. The typical tradeoff is complexity versus performance. It has been well established through extensive research in other fields that asynchronous architectures provide the potential for maximizing performance at the cost of increased complexity<sup>[24]</sup>. It is almost obvious that the same is true on the subject of the AHS separation policy architecture.

## **B.4 SPACING AND CAPACITY EVALUATIONS**

In this section we present briefly the fundamental factors that affect traction during vehicle acceleration and braking.

Traction is what ultimately defines the braking capabilities of any kind of vehicle, under any kind of whether and road conditions. Then we develop likely emergency stopping scenarios for each AHS concept under consideration which we then use to calculate intervehicle spacing and capacity.

#### B.4.1 Adhesion and Friction

The friction force between two surfaces is defined as the force opposing the relative displacement of the two surfaces when a force is applied as shown in Figure B.4.1-1. In the context of vehicle traction this force is referred to as adhesion. Adhesion (attraction between two surfaces) and friction (resistance to relative motion of adjacent surfaces) are very complex physical phenomena. But for practical purposes it is common to use the approximation that the magnitude of the friction force  $F$  depends on two factors only: The normal force  $G$  between the two surfaces and a dimensionless coefficient of friction  $\mu$ , such that:

Eq. 11

$$F = \mu G$$

The value of the coefficient of friction  $m$  depends on the characteristics of the two surfaces, primarily their smoothness and

their hardness, and on the relative speed  $V_r$  between them. For most surfaces, as  $V_r$  increases,  $\mu$  decreases. When the two surfaces do not move  $\mu$  assumes a considerably higher value, referred to as the static friction coefficient.

Applying the general concept to the problem of vehicle traction, it is clear that the maximum Tractive or Braking Effort  $TE_{max}$  which can be utilized is limited by the tire to road surface adhesion.

Eq. 12

$$TE_{max} = \mu G_a$$

where  $G_a$  is the weight on the wheels which apply the force. For propulsion  $G_a$  is the weight on the powered axle while for braking  $G_a$  represents the total vehicle weight  $G$  since the brakes act on all wheels. The actual weight distribution between front and rear axles depends on vehicle design and furthermore varies as a function of the actual deceleration due to the mass transfer phenomenon.

The change of  $\mu$  with speed is very important in traction and friction. It makes braking at high speeds more difficult than at low speeds because it increases the possibility of skidding. Any spinning or skidding of the wheels results in a rapid increase of the relative speed  $V_r$  between the

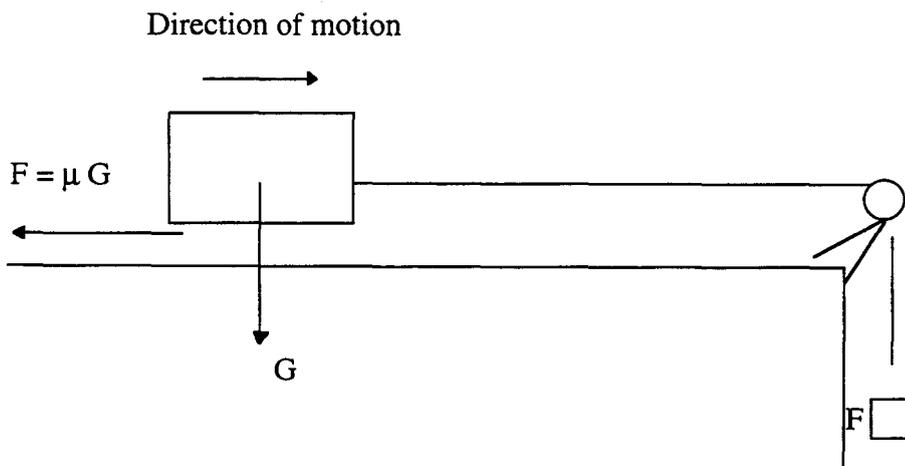


Figure B.4.1-1. Physical Representation of Friction Force F

## Appendix B – Spacing and Capacity Evaluations for Different AHS Concepts

wheels and the road surface and therefore a sudden reduction of  $\mu$ . As a result, traction is lost. To restore the friction coefficient spinning or skidding must be terminated by reducing the tractive or braking effort. This is the principle of operation of the so called Antilock Braking Systems (ABS).

The value of  $\mu$  for highway vehicles depends on the type and condition of the surface. A range of values for most classes of vehicles is shown in Figure B.4.1-2<sup>[8]</sup>.

The braking ability of all vehicles is best on dry pavement. It degrades substantially on wet pavement and braking ability is virtually lost on snow.

In our analysis, we use data from vehicle tests performed by established authorities. For passenger vehicles, we use information from the "Consumer Reports" publication<sup>[9]</sup> and the consumer oriented "Road and Track" magazine<sup>[10]</sup>. For heavy vehicles like buses and trucks, we obtained information

from actual tests<sup>[11]</sup>. Based on these data, we have estimated the braking capabilities of a range of passenger and heavy vehicles on dry, wet and snowed road pavement. In a more or less expected fashion, we found that sports cars can achieve the best braking distances (highest deceleration), followed by middle and upper class medium size vehicles (such as in the "sports sedan" category), followed by small or economy class vehicles. The last finding is a little counter intuitive, based on the fact that small vehicles are light weight thus require less energy dissipation to achieve braking and are less demanding of good tire performance. Yet there is an obvious trend for auto manufacturers to try to match the braking capabilities with the acceleration capabilities of a given vehicle. We found that the trend is to offer approximately double the deceleration (in g's) to the available acceleration (also in g's) in low gear. That's a ball park figure, of course, and deviations do exist.

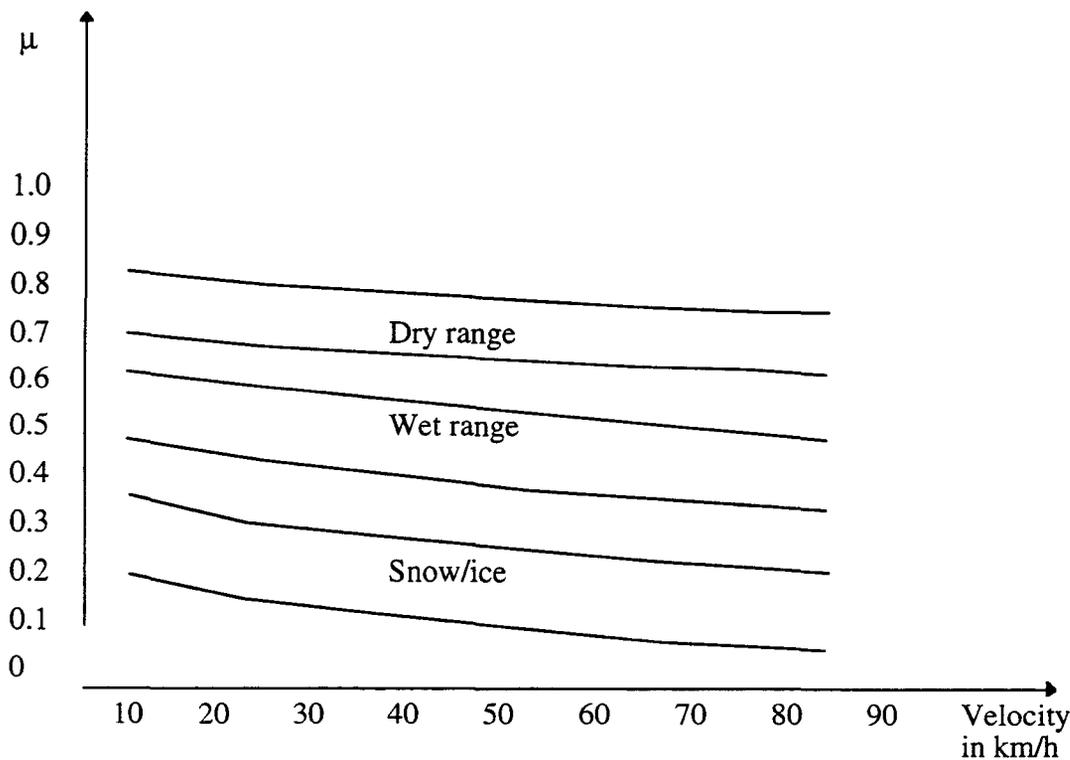


Figure B.4.1-2. Friction Coefficient of Vehicles with Rubber Tires

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The braking capability of any vehicle degrades on wet pavement by a factor determined by the texture of the pavement and the type of tires used. We represent that as a change in the friction coefficient  $\mu$ . The data collected give a quantitative estimate of the friction coefficient on dry, wet and snowed pavement. The numbers of course vary depending on the vehicle, its tires and the presence of ABS. A typical vehicle that can achieve 0.8g deceleration on dry pavement can go down to 0.55g in wet conditions and to as low as 0.15g in snow conditions. The collected braking test results are presented in Appendix A.

In our study, we simplified somewhat our assumptions regarding the friction coefficient  $\mu$ . Instead of assuming a maximum deceleration of 1g and scaling it by the typical value of  $\mu$ , i.e., 0.8 for passenger vehicles, we used the value 0.8g for maximum deceleration and assumed that  $\mu$  is 1.0. This does not affect the results for braking on dry road pavement. Then for wet road conditions we assumed a worst case scenario where the friction coefficient becomes half, i.e.,  $\mu$  becomes 0.5 while the maximum deceleration remains at 0.8g for passenger vehicles. Similarly, instead of assuming different values of  $\mu$  for buses and for heavy trucks, we used the same value for all of them, but we used a different value of maximum deceleration for each class. We used 0.4g maximum deceleration for buses and 0.3g maximum deceleration for heavy trucks. These numbers are based on measurements on actual vehicles, and the data can be found in Appendix A.

The maximum deceleration that each vehicle can achieve depends on many factors and therefore it cannot be predicted exactly. It depends mostly on the tires of course, like the quality and type of tread, hardness, temperature, inflation pressure and the age of the tire. It also depends on the size and type of friction materials in the brakes, the mass distribution of the vehicle, the presence of ABS and many other factors. In our analysis we simplify these complex dependencies by using the abstraction of uniform value of  $\mu$  and assuming appropriate values for maximum

deceleration for different classes of vehicles, without affecting the accuracy of the results.

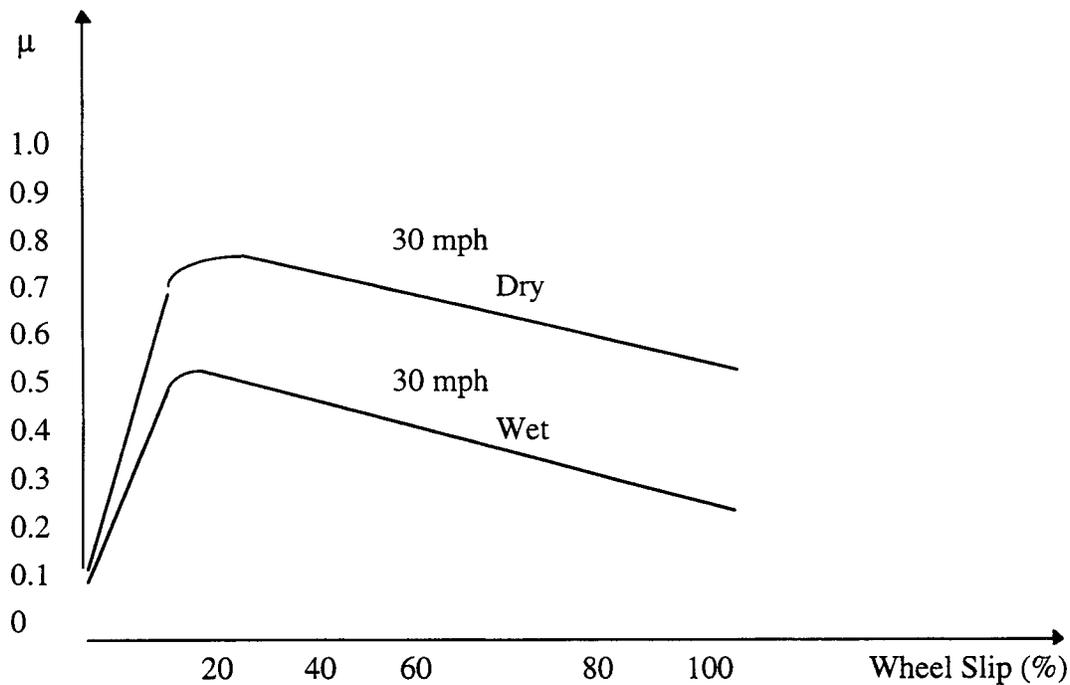
During the emergency braking phase the jerk is not intentionally limited and the maximum deceleration is allowed to be as large as the vehicle can achieve. The jerk clearly depends on the mass of the vehicle first and on the hydraulic brake system second. It clearly depends on the rate of change of the force that the driver applies on the brake pedal in the case of manually driven vehicles. For automated vehicles it will depend on the dynamics of the brake actuator. It would simply be inversely proportional to the mass of the vehicle if all the vehicles had exactly the same actuators and hydraulic systems, but this is certainly not going to be the case.

Based on our experience with an actual brake system which is in use in a prototype automated passenger class vehicle, we made an educated guess for other classes of vehicles. We assumed that the maximum jerk is limited to 50 *meters/sec*<sup>3</sup> for passenger vehicles, 40 *meters/sec*<sup>3</sup> for buses and 30 *meters/sec*<sup>3</sup> for heavy loaded trucks.

### B.4.2 Uniform Versus Non-uniform Braking

For a realistic estimation of the theoretical capacity, we have assumed a “typical” maximum deceleration level for each class of vehicles, based on actual test data. Since discrepancies of 10% or more can be clearly seen in the braking capabilities among vehicles of the same class, we have made the assumption of a 10% discrepancy in maximum deceleration between the leader and the follower in the sense that the follower has inferior maximum deceleration capability, an assumption which inevitably generates the need for more spacing.

To be realistic, this discrepancy exists mostly at the limit of the braking capability of the vehicles, when braking occurs in the unstable region where the slope of  $\mu$  versus wheel slip is negative as seen in Figure B.4.2-1<sup>[7]</sup>. At that point, demanding slightly higher deceleration results in skidding of the tires and in a sharp reduction in the  $\mu$  and in overall deceleration. In our effort to



**Figure B.4.2-1. Braking Coefficient Versus Slip**

represent a realistic worst case scenario, we assumed 10% deviation from the maximum braking capability for the following vehicle in all cases of unrestricted braking, i.e., when the traction of the tires is pushed to the limits. On the other hand, braking by applying less than the maximum deceleration is easier because we can stay away from the unstable region of the  $\mu$  curve. This can be used to our benefit if we impose a limit in deceleration for all vehicles. This limit is a common denominator that all vehicles should be able to meet by a proper design of their control system. This is the definition of the concept we will henceforth call “uniform braking”. By staying away from the unstable braking region we can almost guarantee a better control of the magnitude of the deceleration. This justifies using only 5% deviation from the nominal braking capability for the follower in the case of uniform braking. Uniform braking is more crucial in platooning where, in the interest of efficiency, vehicles within each platoon have to have similar performance. For completeness and for the sake of comparison, we analyzed the effects of

uniform braking both in platooning and non-platooning environments.

The concept that all vehicles should be restricted to a closely matched (i.e. uniform) degree of deceleration is clearly an architectural decision. We assumed that the braking deceleration on a dry road can be restricted to 0.5g for all passenger vehicles, 0.3g for all buses and 0.2g for all heavy trucks. The idea here is to use a number that every vehicle in its respective class can comfortably achieve. This helps guarantee that the deviation from one vehicle to another will be less than 5% in the worst case. So we used a 5% discrepancy in the deceleration of the leading and following vehicle to represent the worst case mismatch in the case of uniform braking.

### B.4.3 Mixing of Vehicle Classes

The mixing of different classes of vehicles on the same AHS will affect capacity due to the different braking capabilities of the different classes of vehicles. In our analysis we consider three different vehicle classes, possessing fundamentally different

characteristics: Passenger vehicles (P), buses (B) and heavy trucks (T).

This leads to the following possible combinations:

- (a) PP: A Passenger vehicle leading a Passenger vehicle
- (b) PB: A Passenger vehicle leading a Bus
- (c) PT: A Passenger vehicle leading a Truck
- (d) BP: A Bus leading a Passenger vehicle
- (e) BB: A Bus leading a Bus
- (f) BT: A Bus leading a Truck
- (g) TP: A Truck leading a Passenger vehicle
- (h) TB: A Truck leading a Bus
- (i) TT: A Truck leading a Truck

We made the following distinctions in mixing possibilities:

- a) No mixing.

Traffic consisting of passenger vehicles only, with 0% mixing of other vehicle classes among the passenger vehicles. In this case, the passenger vehicle to passenger vehicle (PP) minimum headway was assumed between all vehicles.

- b) Allowed mixing of vehicle classes.

All cases of mixing assume uniform mixing, i.e., the minority vehicles are uniformly distributed among the population of passenger cars. This is a realistic assumption as long as the percentage of mixing is fairly low.

#### Case 1:

Traffic consisting of passenger vehicles with 5% mixing of buses. In this case, the passenger vehicle to passenger vehicle (PP) minimum headway was assumed between 90% of the vehicles, passenger vehicle to bus (PB) minimum headway between 5% of the vehicles and bus to passenger vehicle (BP) between 5% of the vehicles.

#### Case 2:

Traffic consisting of passenger vehicles with 5% mixing of trucks. In this case, the passenger vehicle to passenger vehicle (PP) minimum headway was assumed between 90% of the vehicles, passenger vehicle to truck (PT) minimum headway between 5% of the vehicles and truck to passenger vehicle (TP) between 5% of the vehicles.

#### Case 3:

Traffic consisting of passenger vehicles with 2.5% mixing of buses and 2.5% mixing of trucks. In this case, the passenger vehicle to passenger vehicle (PP) minimum headway was assumed between 90% of the vehicles, passenger vehicle to bus (PB) minimum headway between 2.5% of the vehicles passenger vehicle to truck (PT) minimum headway between 2.5% of the vehicles bus to passenger vehicle (BP) between 2.5% of the vehicles. and truck to passenger vehicle (TP) between 2.5% of the vehicles.

#### Case 4:

Traffic consisting of passenger vehicles with 5% mixing of buses. and 5% mixing of trucks. In this case, the passenger vehicle to passenger vehicle (PP) minimum headway was assumed between 80% of the vehicles, passenger vehicle to bus (PB) minimum headway between 5% of the vehicles passenger vehicle to truck (PT) minimum headway between 5% of the vehicles bus to passenger vehicle (BP) between 5% of the vehicles. and truck to passenger vehicle (TP) between 5% of the vehicles.

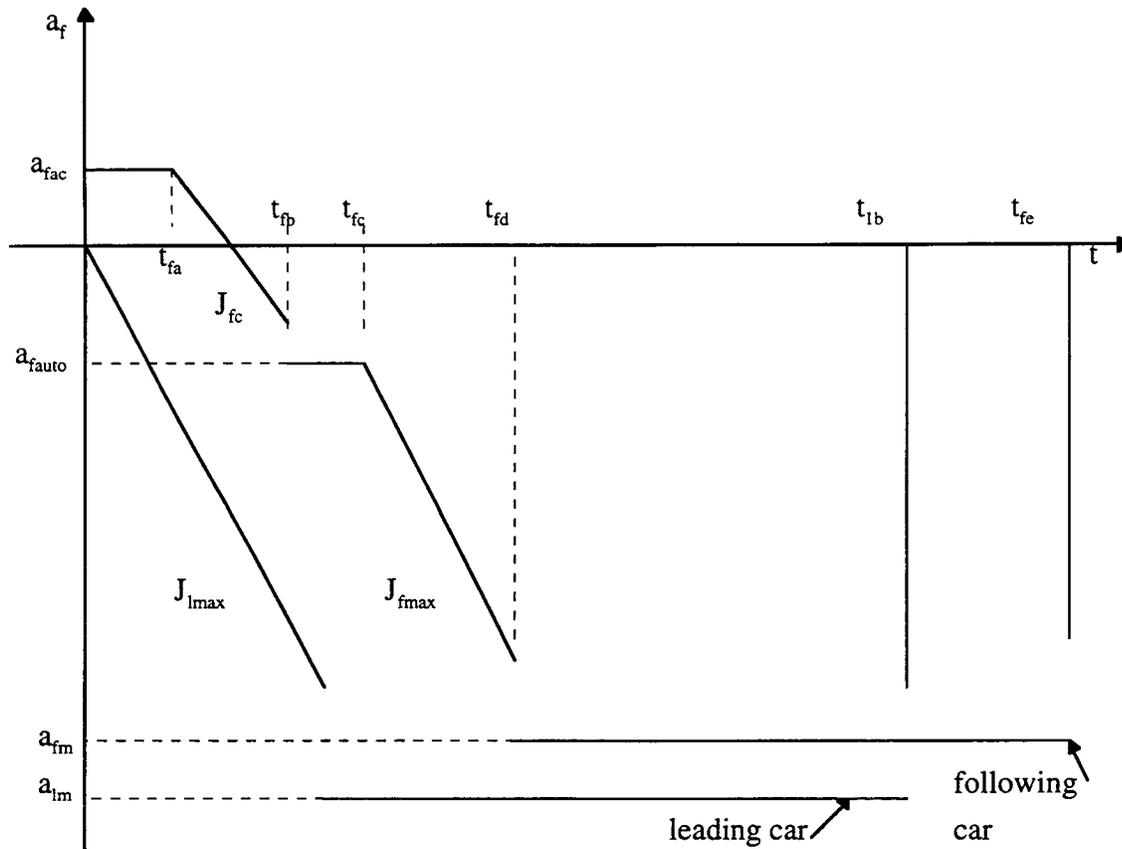
### B.4.4 Autonomous Vehicles

In the case of autonomous vehicles, each vehicle relies on its own sensors to determine the motion intentions of the leading vehicle. Since there is no vehicle to vehicle communication, each vehicle has to

## Appendix B – Spacing and Capacity Evaluations for Different AHS Concepts

use relative speed and spacing measurements to determine the intentions of the vehicle ahead. Therefore, in calculating a safe intervehicle spacing we consider the following worst case stopping scenario.

The acceleration (actually deceleration) profile of the leading and following vehicles involved in a braking maneuver is assumed to follow the trajectories shown in Figure B.4.4-1.



**Figure B.4.4-1. Autonomous Vehicles**

The leading vehicle performs emergency braking at time  $t = 0$ , at a maximum rate of change (jerk) equal to  $J_{lmax}$  until it reaches a maximum deceleration of  $a_{lm}$ . The follower, which might have been accelerating initially, at  $a_{fac}$  starts decelerating after a detection and brake actuation delay equal to  $t_{fa}$  in an effort to maintain the desired spacing. Since initially the follower is not aware that the leader is performing emergency braking, it limits its jerk and deceleration to  $J_{fc}$  and  $a_{fauto}$  respectively, in an effort to meet the vehicle control objective and at the same time maintain passenger comfort. The follower initiates emergency braking at  $t = t_{fc}$ . At this

time passenger comfort is no longer a crucial issue and braking is done with maximum jerk  $J_{fmax}$  and maximum deceleration  $a_{fm}$ .

In this paper we use the above stopping scenario to calculate the minimum time headway for collision free vehicle following by substituting appropriate numerical values for all the above parameters.

In evaluating the above scenario we adopted a set of likely initial conditions at the onset of braking. The assumptions regarding the initial conditions are the following: The leader has been traveling at a speed of

60 miles per hour while the follower has an instantaneous velocity 5% higher, i.e. 63 miles per hour and an instantaneous acceleration  $a_{fac} = 0.15g$ . These conditions represent the realistic scenario that the follower had been performing a position adjustment as in trying to catch up with the leader. Therefore the vehicle is accelerating just before it has to start braking. When the vehicle detects that the leader is braking (which involves a 0.1 sec delay for detection and a 0.1 sec delay in the actuator) it starts braking until it reaches the maximum allowable deceleration  $a_{fauto} = -0.1g$  for passenger comfort.

The vehicle initially applies a limited amount of braking because at the onset of braking it is not known if the leader is simply slowing down or performing emergency braking. If the follower applies emergency braking every time it detects the leader slowing down it would be detrimental to the stability of the traffic flow. Therefore the follower applies limited braking at first, with the objective of not upsetting the quality of the ride of the passengers or the position and velocity error of any vehicles behind. For this reason, the Jerk is limited to 5 meters/sec<sup>3</sup> during this phase.

Eventually, the follower will detect that the headway is diminishing rapidly and therefore the leader is performing an emergency braking maneuver. We assumed that the detection of emergency braking involves 0.3 seconds of delay.

Using these parameter values, we computed the necessary headways different road conditions and levels of mixing of classes of vehicles. The spacing results are presented in Table B-1 for the case of dry road surface. The spacing results for the case of wet road surface are presented in Table B-2.

The spacing calculations in Tables 1 and 2 are based on the assumption that vehicles can brake with maximum possible deceleration depending on their capabilities. Another possible scenario is to use the concept of uniform braking that limits the maximum deceleration and maximum jerk to values that could be met and used by all vehicles of the same class. These limits will

make the braking performance of the vehicles very similar. Using this scenario we calculated spacings based on the vehicle values shown in Table B-3. In this case due to uniformity we assume 5% deviation between decelerations of vehicles of the same class. This 5% deviation accounts for inaccuracies in measuring acceleration/ deceleration and maintaining the desired one using the on board vehicle controller.

Based on the above spacings the maximum possible throughput referred to as the capacity  $C$  measured as the number of vehicles per hour per lane is given by the formula

Eq. 13

$$C = (360000V)[(100-2W_T-2W_B)(L_p+h_{pp}V) + W_T(L_p+h_{pT}V+h_{TP}V+L_T) + W_B(L_p+h_{pB}V+h_{BP}V+L_B)]^{-1}$$

where  $V$  is the speed of flow measured in meters/sec,  $L_p$  is the length of passenger cars,  $L_B$  is the length of buses and  $L_T$  is the length of trucks with trailers, in meters. The parameter  $h_{pp}$  is the minimum time headway between passenger cars,  $h_{pT}$  is the minimum time headway between a passenger car and a truck that follows it,  $h_{TP}$  is the minimum time headway between a truck and a passenger car that follows it,  $h_{pB}$  is the minimum time headway between a passenger car and a bus that follows it and  $h_{BP}$  is the minimum time headway between a bus and a passenger car that follows it, in seconds.  $W_B$  is the percentage of buses and  $W_T$  is the percentage of trucks in the mix. We use eq. (13) and the numerical results of Tables B-1, B-2 and B-3 to calculate the capacity values which are presented in Table B-4A.

In eq. 13 we assumed that a bus or a truck is always between two passenger vehicles and the passenger vehicle recognizes when its leader is a truck or a bus. This is a reasonable assumption because the radar sensors used for ranging measurements can be equipped with the feature of being able to distinguish different classes of vehicles. Without this assumption each vehicle has to assume the worst possible situation which is the one where each vehicle treats its leader

as a passenger vehicle i.e., a vehicle with the highest possible braking capability. In this case eq. 13 is modified to

Eq. 14

$$C = (360000V)[(100-2W_T-2W_B)(L_P+h_{PP}V) + W_T(L_P+h_{PT}V+h_{PP}V+L_T) + W_B(L_P+h_{PB}V+h_{PP}V+L_B)]^{-1}$$

The capacity results for this case are listed in Table B-4B.

### B.4.5 Free Agent Vehicles - Infrastructure Supported

In the case of Free Agent Vehicles we assumed the braking scenario shown in Figure B.4.5-1. The use of vehicle to

vehicle communication simplifies the task of determining when the leading vehicle is performing emergency braking. The leader at  $t = 0$  starts performing emergency braking. At  $t = 0$  it communicates its intention to the following vehicle. The following vehicle receives the information from the leader and verifies using its own sensors that it has to perform an emergency braking as well.

The assumptions regarding the initial conditions are the same as in the previous case: We assume the leader has been traveling at a speed of 60 miles per hour while the follower has an instantaneous velocity of 63 miles per hour and an instantaneous acceleration of 0.15g, as if the follower had been trying to catch up with the leader.

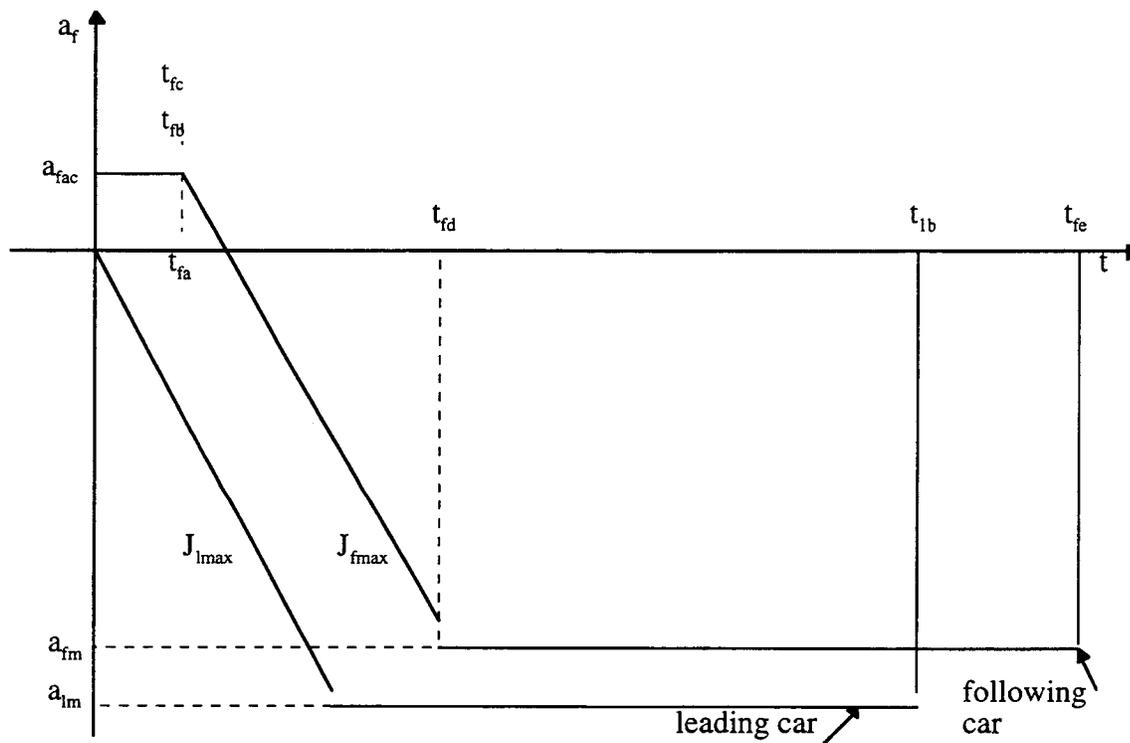


Figure B.4.5-1. Infrastructure Supported Free Agent Vehicles

When the vehicle detects the leader is braking and at the same time receives the information that this is emergency braking,

it bypasses the limited jerk/limited braking stage shown in Figure B.4.5-1 in the previous section. In Figure B.4.4-1, we

have clustered the detection and the actuation delay into a single 0.1 seconds delay before the follower applies emergency braking. In effect, the actuation delay is compensated for by the fact that the vehicle knows in advance it will have to apply the brakes, and the brake actuator may be pre-loaded. Therefore in Figure B.4.5-1 we assume  $t_{fa} = 0.1$  sec and  $t_{fc} = 0.1$  sec. The minimum headway results together with the numerical values of the variables shown in Figure B.4.5-1 are presented in Tables B-5, B-6 and B-7. Equation (13) is used to calculate capacity for different levels of mixing of different classes of vehicles. The results are shown in Table B-8.

#### **B.4.6 Free Agent Vehicles - Infrastructure Managed**

In the case of Free Agent Vehicles with infrastructure management we have assumed that the infrastructure has the primary responsibility of detecting the presence of emergencies and synchronizing the onset of emergency braking of all vehicles involved. This results in the most favorable timing for braking delays.

The infrastructure may simply issue the command "Begin emergency braking now" and all vehicles receiving this will have to apply maximum braking without further delay. This, not only simplifies the task of determining when the leading vehicle is performing emergency braking but also minimizes the relative delay in propagating the onset of emergency braking from each vehicle to the vehicle behind, effectively down to zero.

We have listed the actuation delay as a single 0.1 seconds delay before each vehicle applies emergency braking, but since all the vehicles receive the command at the same time the relative delay is zero and this is reflected in the value of the parameter  $t_{fc}$ . The time  $t_{fc}$  represents the total delay between the onset of emergency braking between the leader and the follower and in this case  $t_{fc} = 0$ .

The assumptions regarding the initial conditions are the same as before: The leader has been traveling at a speed of 60

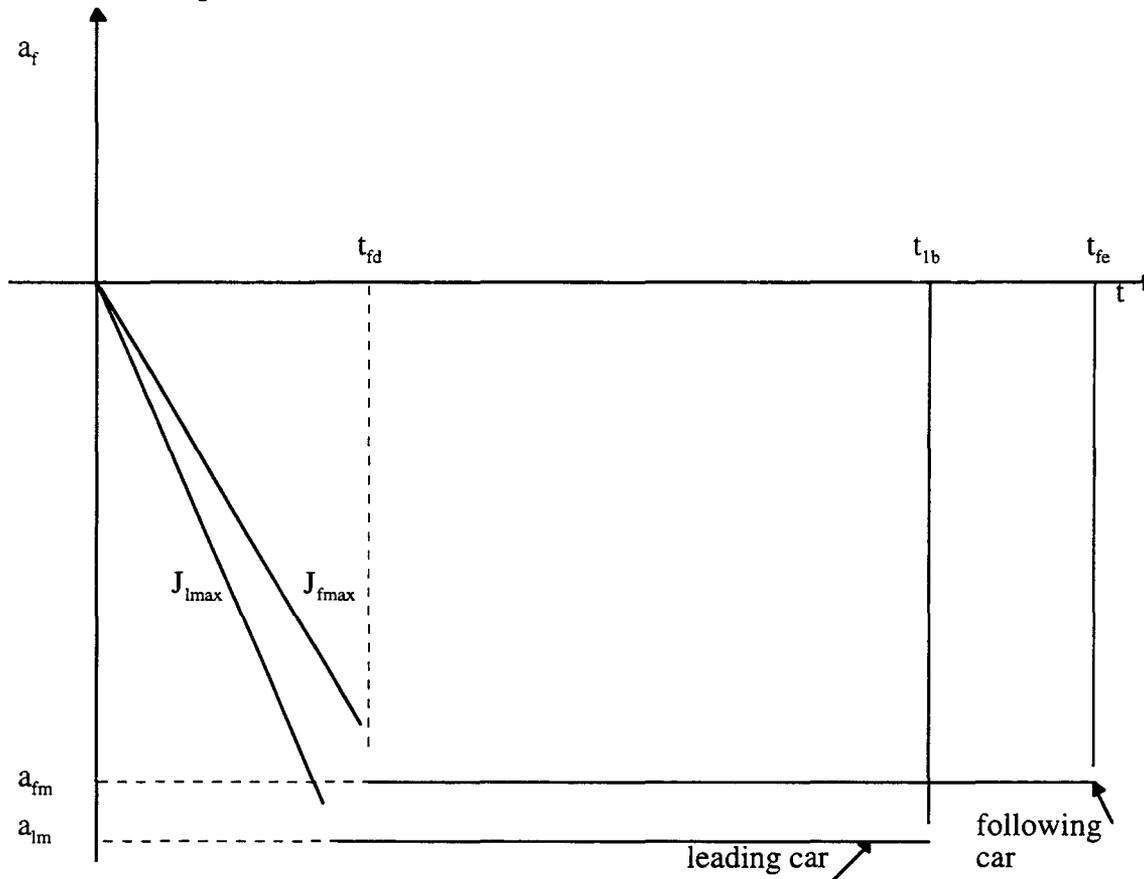
miles per hour while the follower has an instantaneous velocity of 63 miles per hour and an instantaneous acceleration of 0.15g, as if the follower had been trying to catch up with the leader. The minimum headway results together with the numerical values of the variables shown in Figure B.4.6-1 are presented in Tables B-9, B-10 and B-11. Equation (13) is used to calculate capacity for different levels of mixing of different classes of vehicles. The results are shown in Table B-12.

#### **B.4.7 Vehicles Platoons Without Coordinated Braking**

In the platooning without coordinated braking case, we have assumed that each vehicle notifies the vehicle behind about its braking capabilities and the magnitude and timing of the braking force used.

When the platoon leader detects an emergency, it immediately notifies the vehicle that follows. There will be a delay while the message propagates from each vehicle to the vehicle behind, as well as an actuation delay. But the actuation delay is not affecting the scenario as long as it is approximately the same for each vehicle. We have assumed that the total delay is 0.1 seconds for every vehicle and it is represented by the parameter  $t_{fa}$ . Therefore we have accounted for only a 0.1 seconds total delay in propagating the message from each vehicle to the vehicle behind and this becomes the value of the parameter  $t_{fc}$ , which represents the delay of the onset of emergency braking.

The assumptions regarding initial conditions are as follows: The leader has been traveling at a speed of 60 miles per hour while the follower has an instantaneous velocity of 61.5 miles per hour. Since the platoon protocol involves a much tighter control of individual vehicle velocity than in the case of free agents, only a 2.5% difference is assumed in the initial vehicle velocities. The instantaneous acceleration was also taken to be 0g as it would be impossible for a vehicle in a platoon to be accelerating while the vehicle ahead is maintaining constant speed. Both the velocities and the accelerations of vehicles



**Figure B.4.6-1. Infrastructure Managed Free Agent Vehicles**

in platoons are expected to be closely coordinated. In addition, for reasons explained earlier we assumed no mixing of vehicle classes.

The inter-platoon spacing depends on the concept used for platoon following. We compared three different concepts.

- a) Autonomous platoons, where platoons do not communicate with each other and each platoon relies on its own sensors to detect the motion of a leading platoon. In this case, the inter-platoon spacing is calculated as in the case of autonomous vehicles. Therefore, each vehicle assumes  $t_{fc} = 0.1$  seconds and each platoon entity assumes the parameters of autonomous vehicles:  $t_{fc} = 0.3$  seconds for 10 car platoons and again  $t_{fc} = 0.3$  seconds for 20 car platoons.
- b) Free agent platoons supported by the infrastructure where the inter-platoon spacing is calculated as in the case of free agent vehicles with infrastructure support. Each vehicle in the platoon assumes  $t_{fc} = 0.1$  seconds. Each platoon entity assumes the parameters of free agent infrastructure supported vehicles:  $t_{fc} = 0.1$  seconds for 10 car platoons and  $t_{fc} = 0.1$  seconds for 20 car platoons.
- c) Free agent platoons managed by the infrastructure where the inter-platoon spacing is calculated as in the case of free agent vehicles with infrastructure management. Each vehicle in the platoon assumes  $t_{fc} = 0.1$  seconds. Each platoon entity assumes the parameters of free agent infrastructure managed vehicles:  $t_{fc} = 0$  seconds for

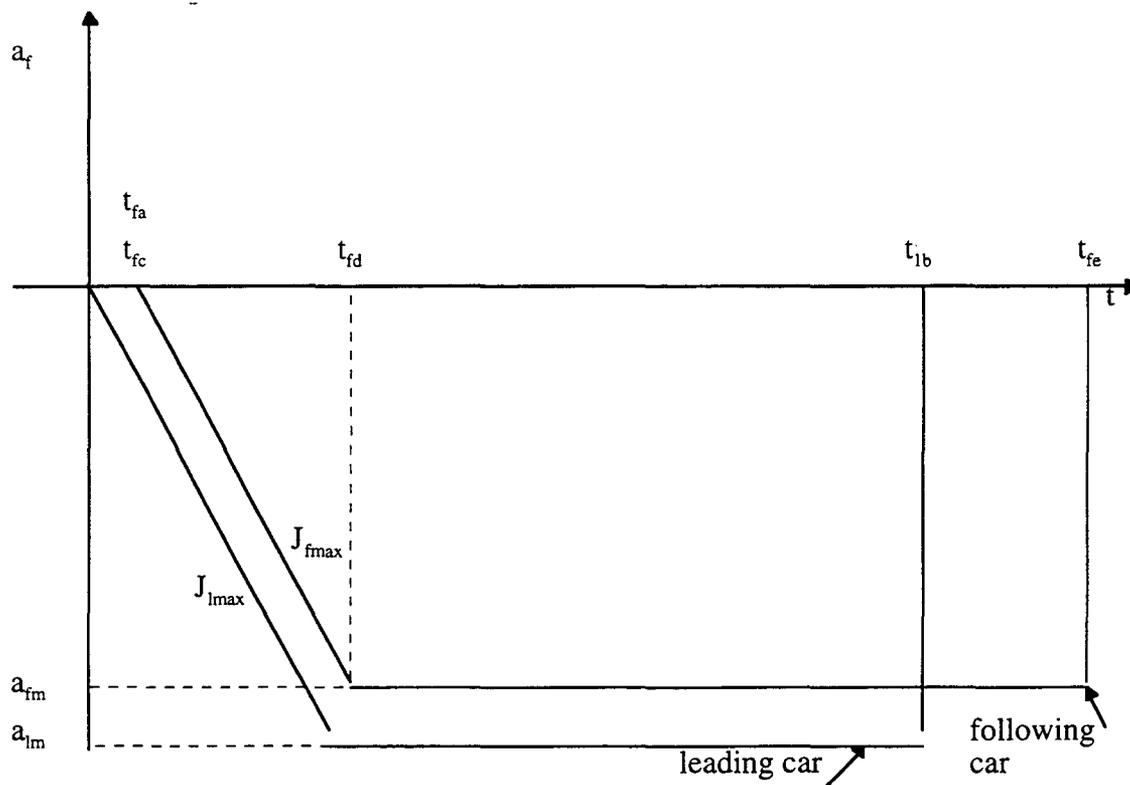


Figure B.4.6-1. Platoons Without Coordinated Braking

10 car platoons and  $t_{fc} = 0$  seconds for 20 car platoons.

The capacity is calculated in each case using the equation:

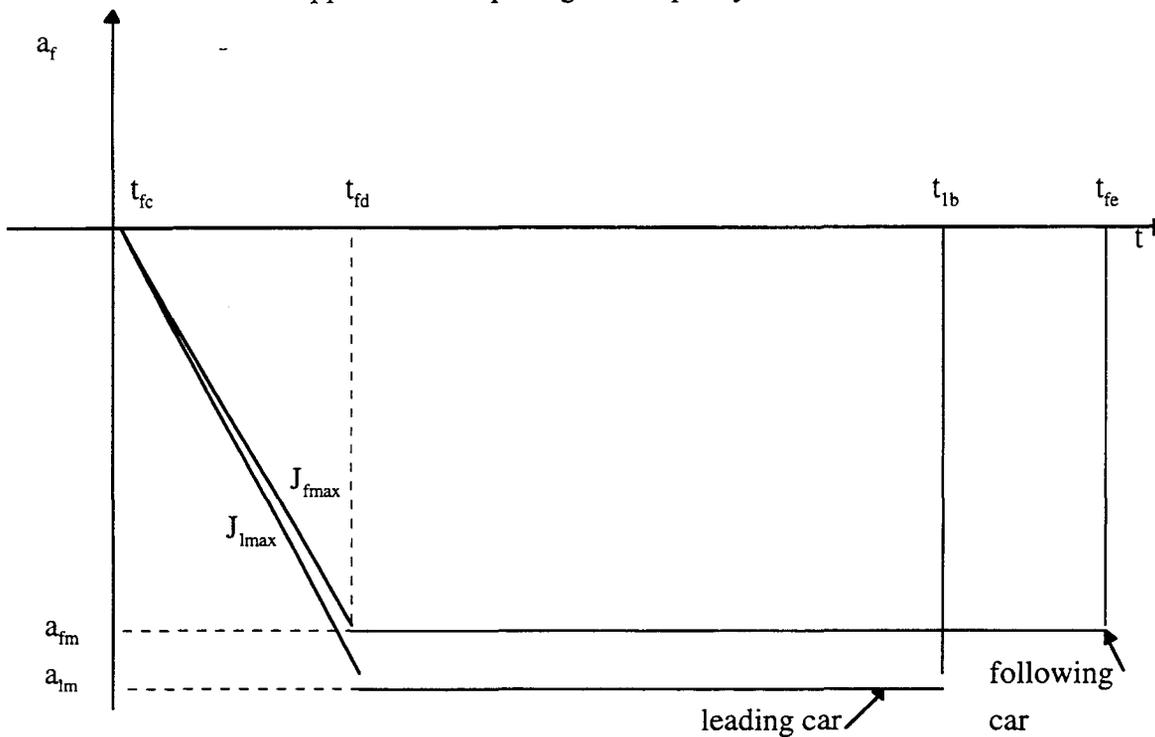
Eq. 15

$$C = \frac{(3600 V N)}{(h_{pp} V + L_p) (N-1) + H_{pp} V + L_p}$$

where  $L_p$  is the length of each vehicle in the platoon (we have assumed vehicles of same length),  $h_{pp}$  is the intra-platoon time headway,  $H_{pp}$  is the inter-platoon time headway and  $N$  is the number of vehicles in the platoon. The resulting intra-platoon spacing for platoons without coordinated braking can be found in Table B-13. The capacity results are presented in Table B-14.

#### B.4.8 Vehicle Platoons With Coordinated Braking No Delay

In platooning with coordinated braking we assume that the vehicle in the platoon leader position assumes the primary responsibility of detecting emergencies and notifying each and every vehicle in the platoon. This notification takes place through a network style vehicle to vehicle communications system that minimizes the communication delays. The platoon leader notifies all the vehicle in the platoon about the magnitude of the braking force that is to be applied and also the exact time this is to be applied. This architecture, not only eliminates the need for each vehicle to detect the magnitude of braking and if the braking should be limited or emergency braking, but also can adjust the onset of emergency braking for an effective 0 seconds relative delay, or even to an artificial negative relative delay.



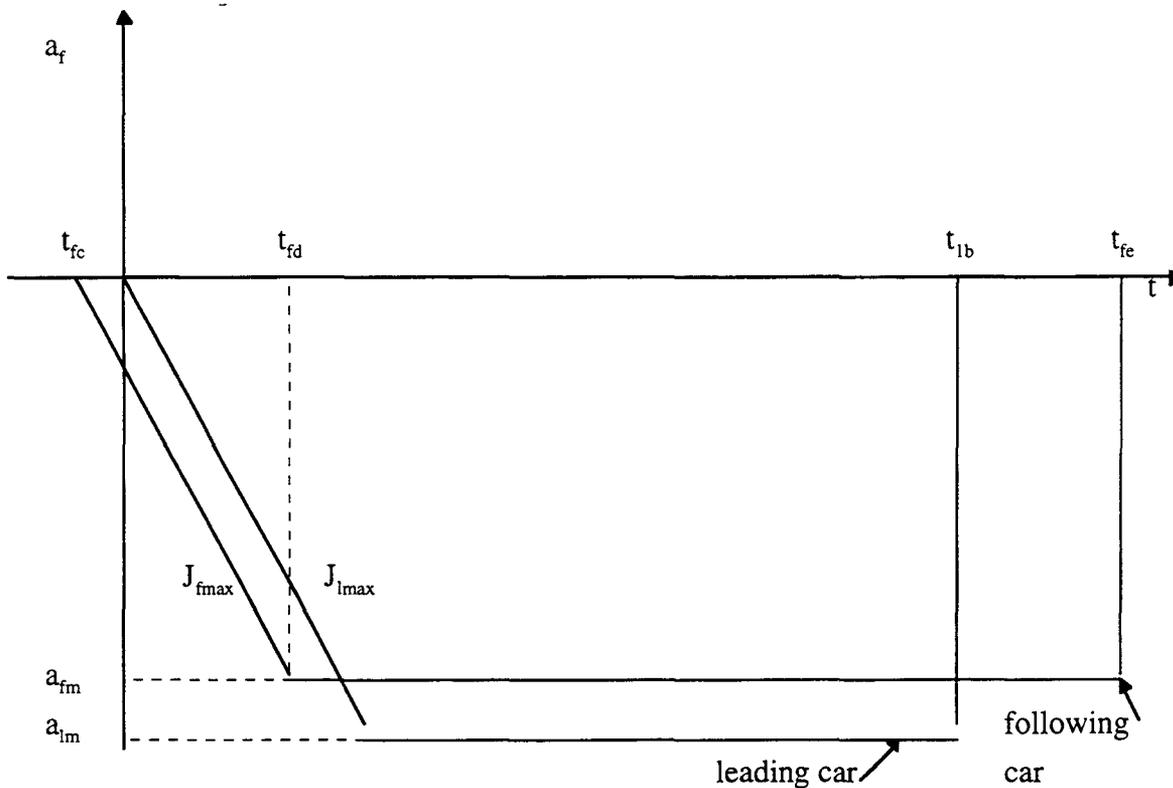
**Figure B.4.7-1. Platoons with Coordinated Braking and No Delay**

The brake actuation delay can be completely compensated for and it is not affecting the scenario as long as it is approximately the same for each vehicle. We have assumed it is 0.1 seconds on every vehicle. Therefore we have made the assumption of exactly 0 seconds total delay for the onset of braking for each vehicle in the platoon and this is the value of the parameter  $t_{fc}$  which represents this delay.

The other assumptions regarding the initial conditions are the same as in all architectures involving platoons. The leader has been traveling at a speed of 60 miles per hour while the follower has an instantaneous velocity of 61.5 miles per hour. The instantaneous acceleration was also take to be 0g as it would be impossible for a vehicle in a platoon to be accelerating while the vehicle ahead is maintaining constant speed. Both the velocities and the accelerations of vehicles in platoons are expected to be closely coordinated.

For the inter-platoon spacing we used and compared three different concepts.

- a) Autonomous platoons where the inter-platoon spacing is calculated as in the case of autonomous vehicles. Therefore, each vehicle assumes  $t_{fc} = 0$  seconds and each platoon entity assumes the parameters of autonomous vehicles:  $t_{fc} = 0.3$  seconds for 10 car platoons and again  $t_{fc} = 0.3$  seconds for 20 car platoons.
- b) Free agent platoons supported by the infrastructure where the inter-platoon spacing is calculated as in the case of free agent vehicles with infrastructure support. Each vehicle in the platoon assumes  $t_{fc} = 0$  seconds. Each platoon entity assumes the parameters of free agent infrastructure supported vehicles:  $t_{fc} = 0.1$  seconds for 10 car platoons and  $t_{fc} = 0.1$  seconds for 20 car platoons.
- c) Free agent platoons managed by the infrastructure where the inter-platoon



**Figure B.4.8-1. Platoons with Coordinated Braking with Staggered Delay**

spacing is calculated as in the case of free agent vehicles with infrastructure management. Each vehicle in the platoon assumes  $t_{fc} = 0$  seconds. Each platoon entity assumes the parameters of free agent infrastructure managed vehicles:  $t_{fc} = 0$  seconds for 10 car platoons and  $t_{fc} = 0$  seconds for 20 car platoons.

The inter-platoon spacing results for platoons with coordinated braking are calculated using equation (15), based on the intra-platoon spacings presented in Table B-15. The capacity results are presented in Table B-16.

#### **B.4.9 Vehicle Platoons with Coordinated Braking and Staggered Timing**

This case is identical to the previous one except for the purposeful timing of the onset of emergency braking. In the platooning with coordinated braking case we have

assumed the vehicle in the platoon leader position assumes the primary responsibility of detecting emergencies and notifying each and every vehicle in the platoon. This notification takes place through a network style vehicle to vehicle communications system that minimizes the communication delays. The platoon leader notifies all the vehicle in the platoon about the magnitude of the braking force that is to be applied and also the exact time this is to be applied. This architecture, not only eliminates the need for each vehicle to detect the magnitude of braking and if the braking should be limited or emergency braking, but also can adjust the onset of emergency braking to an artificial negative relative delay.

Therefore we have made the choice of using a 0.1 seconds total delay for the onset of braking for each vehicle in the platoon going from the tail to the head, in the sense that the tail of the platoon is requested to brake first,

then the vehicle ahead after a delay of 0.1 seconds, until the command to begin braking becomes effective for the platoon leader. Therefore we used a negative value, -0.1 seconds, as the value of the parameter  $t_{fc}$  which represents the relative delay for two consecutive vehicles within the platoon.

We cannot omit mentioning the fact that the platoon leader which detects the presence of emergency is subsequently restrained from braking until every other vehicle in the platoon has begun braking. Therefore, while this architecture allows to minimize the necessary spacing between vehicles in the platoon, it increases the inter-platoon spacing requirement.

The other assumptions regarding the initial conditions are the same for all architectures involving platoons. For the inter-platoon spacing we used and compared several different concepts.

- a) Autonomous platoons where the inter-platoon spacing is calculated as the sum of the inter-vehicle spacing used in the case of autonomous vehicles and the product of the coordinated braking delay with the number of vehicles in a platoon. Each vehicle in the platoon assumes  $t_{fc} = -0.1$  seconds. Each platoon entity assumes  $t_{fc} = 1.3$  seconds for 10 car platoons and  $t_{fc} = 2.3$  seconds for 20 car platoons.
- b) Free agent platoons supported by the infrastructure where the inter-platoon spacing is calculated as the sum of the inter-vehicle spacing used in the case of free agent vehicles with infrastructure support and the product of the coordinated braking delay with the number of vehicles in a platoon. Each vehicle in the platoon assumes  $t_{fc} = -0.1$  seconds. Each platoon entity assumes  $t_{fc} = 1.1$  seconds for 10 car platoons and  $t_{fc} = 2.1$  seconds for 20 car platoons.
- c) Free agent platoons managed by the infrastructure where the inter-platoon spacing is calculated as the sum of the inter-vehicle spacing used in the case of free agent vehicles with infrastructure management and the

product of the coordinated braking delay with the number of vehicles in a platoon. Each vehicle in the platoon assumes  $t_{fc} = -0.1$  seconds.

Each platoon entity assumes  $t_{fc} = 1.0$  seconds for 10 car platoons and  $t_{fc} = 2.0$  seconds for 20 car platoons.

The capacity is calculated using the following formula:

Eq. 16

$$C = (3600 V N) / [(h_{pp} V + L_p) (N-1) + L_p + (H_{pp} + N t_b) V]$$

where  $L_p$  is the length of each vehicle in the platoon (we have assumed vehicles of same length),  $h_{pp}$  is the intra-platoon time headway,  $H_{pp}$  is the inter-platoon time headway,  $N$  is the number of vehicles in the platoon and  $t_b$  is the coordinated braking delay. The spacing is calculated using equation (16) based on the intra-platoon spacings given in Table B-17. The capacity results are presented in Table B-18.

#### B.4.10 Infrastructure Managed Slotting

The infrastructure managed slotting concept involves a different set of assumptions and parameters. We have not presented it in detail in the tables, except one table which shows capacity estimates under this architecture concept. We used the spacing data for passenger cars by assuming a doubling of all communication delays with an additional 3 meters to account for position inaccuracy, due to the inability to utilize space effectively by using the exact slot size for each vehicle. We also assumed that the follower has no initial acceleration. The capacities computed under these assumptions can be found in Table B-19.

### B.5 DISCUSSION AND CONCLUSIONS

The capacity estimates for each concept considered are summarized in Table B-20. These results indicate that the capacity is reduced by 30% to 40% by going from dry road to wet road conditions under each

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concept. The capacity is also reduced by about 10% if all vehicles are required to use lower but similar braking force during emergency stopping. Mixing of different classes of vehicles reduces capacity by about 11% for 2.5% buses and 2.5% trucks and by about 23% for 5% buses and 5% trucks. Platooning with coordinated braking gives the highest capacities. infrastructure managed slotting gives the lowest. The use of vehicle to vehicle communication for notifying vehicles about the onset of braking used in the Free Agent and Platooning based concepts helps increase capacity considerably.

### B.6 SYMBOLS AND NOTATION

PP:	Passenger car leader, Passenger car follower	$h_{BB}$ :	Minimum time headway between Bus leader, Bus follower, in seconds
PB:	Passenger car leader, Bus follower	$h_{BT}$ :	Minimum time headway between Bus leader, Truck follower, in seconds
PT:	Passenger car leader, Truck follower	$h_{TP}$ :	Minimum time headway between Truck leader, Passenger car follower, in seconds
BP:	Bus leader, Passenger car follower	$h_{TB}$ :	Minimum time headway between Truck leader, Bus follower, in seconds
BB:	Bus leader, Bus follower	$h_{TT}$ :	Minimum time headway between Truck leader, Truck follower, in seconds
BT:	Bus leader, Truck follower	$V_{lo}$ :	Leading Vehicle initial Velocity, in miles per hour.
TP:	Truck leader, Passenger car follower	$V_{fo}$ :	Following Vehicle initial Velocity, in miles per hour.
TB:	Truck leader, Bus follower	$A_{lm}$ :	The maximum achievable deceleration of the leading vehicle in g
TT:	Truck leader, Truck follower	$A_{fm}$ :	The maximum achievable deceleration of the following vehicle in g
$L_P$ :	Length of a passenger vehicle, in meters	$J_{lmax}$ :	The maximum achievable jerk of the leading vehicle in meters/sec <sup>3</sup>
$L_B$ :	Length of a bus, in meters	$J_{fmax}$ :	The maximum achievable jerk of the following vehicle in meters/sec <sup>3</sup>
$L_T$ :	Length of a truck with trailer, in meters	$\mu_{lmax}$ :	The maximum road-tire friction coefficient (dimensionless)
$h_{pp}$ :	Minimum time headway between Passenger car leader Passenger car follower, in sec.	$\mu_{fmax}$ :	The maximum road-tire friction coefficient (dimensionless)
$h_{pb}$ :	Minimum time headway between Passenger car leader, Bus follower, in seconds	$A_{fauto}$ :	The acceleration value under automatic brake control during soft braking, in g
$h_{pt}$ :	Minimum time headway between Passenger car leader, Truck follower, in seconds	$A_{fac}$ :	The initial acceleration value during vehicle following, in g
$h_{bp}$ :	Minimum time headway between Bus leader, Passenger car follower, in seconds	$J_{fc}$ :	The jerk value under automatic brake control during soft braking, in meters/sec <sup>3</sup>
		$t_{fa}$ :	Detection and brake actuation delay applicable to the following vehicle, in seconds.

$t_{fc}$ : The time at which the following vehicle starts the emergency braking maneuver, in seconds

vehicles volume 1. Technical Report no. DOT HS 806 738, April 1985

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### APPENDIX B.1 VEHICULAR DATA REFERENCES

Braking performance comparisons of popular passenger vehicles on dry and wet roads. (from Consumer Reports, March 1995) (Family sedans)						
	Dry			Wet		
	Initial Velocity	Stopping Distance	Deceler/n (avg. g)	Initial Velocity	Stopping Distance	Deceler/n (avg. g)
Chrysler Cirrus Lxi	60 mph	145 ft	0.83 g	60 mph	167 ft	0.72 g
Mercury Mystique LS	60 mph	140 ft	0.86 g	60 mph	165 ft	0.73 g
Ford Contour GL	60 mph	148 ft	0.81 g	60 mph	158 ft	0.76 g
Honda Accord LX	60 mph	143 ft	0.84 g	60 mph	175 ft	0.69 g

Braking performance comparisons on Dry and Wet roads of popular passenger vehicles (from Consumer Reports, May 1995) (Upscale sedans)						
	Dry			Wet		
	Initial Velocity	Stopping Distance	Deceler/n (avg. g)	Initial Velocity	Stopping Distance	Deceler/n (avg. g)
Toyota Avalon XLS	60 mph	129 ft	0.93 g	60 mph	146 ft	0.82 g
Mazda Millenia S	60 mph	136 ft	0.88 g	60 mph	157 ft	0.77 g
Lexus ES300	60 mph	133 ft	0.90 g	60 mph	167 ft	0.72 g
Oldsmobile Aurora	60 mph	136 ft	0.88 g	60 mph	155 ft	0.78 g

Braking performance comparisons on Dry and Wet roads of popular passenger vehicles (from Consumer Reports, June 1995) (Low-Priced Sedans)						
	Dry			Wet		
	Initial Velocity	Stopping Distance	Deceler/n (avg. g)	Initial Velocity	Stopping Distance	Deceler/n (avg. g)
Mazda Protege ES	60 mph	135 ft	0.89 g	60 mph	167 ft	0.72 g
Chevrolet Cavalier LS	60 mph	133 ft	0.90 g	60 mph	165 ft	0.73 g
Nissan Sentra GXE	60 mph	142 ft	0.85 g	60 mph	158 ft	0.76 g
Saturn SL2	60 mph	138 ft	0.87 g	60 mph	157 ft	0.77 g

Braking performance comparisons on Dry and Wet roads of popular passenger vehicles (from Consumer Reports, July 1995) (Mid-Sized Coupes)						
	Dry			Wet		
	Initial Velocity	Stopping Distance	Deceler/n (avg. g)	Initial Velocity	Stopping Distance	Deceler/n (avg. g)
Dodge Avenger ES	60 mph	129 ft	0.93 g	60 mph	157 ft	0.77 g
Ford Thunderbird LX	60 mph	131 ft	0.92 g	60 mph	153 ft	0.79 g
Chevrolet Monte Carlo Z34	60 mph	139 ft	0.87 g	60 mph	165 ft	0.73 g
Buick Riviera	60 mph	133 ft	0.90 g	60 mph	147 ft	0.82 g

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<b>Braking performance comparisons on Dry and Wet roads of popular passenger vehicles (from Consumer Reports, August 1995) (Sport-utility vehicles)</b>						
	<b>Dry</b>			<b>Wet</b>		
	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>
Ford Explorer	60 mph	148 ft	0.81 g	60 mph	181 ft	0.66 g
Jeep Grand Cherokee	60 mph	144 ft	0.84 g	60 mph	159 ft	0.76 g
Chevrolet Blazer	60 mph	156 ft	0.77 g	60 mph	172 ft	0.70 g
Land Rover Discovery	60 mph	143 ft	0.84 g	60 mph	202 ft	0.60 g

<b>Braking performance comparisons on Dry and Wet roads of popular passenger vehicles (from Consumer Reports, September 1995) (Small, Cheap Cars)</b>						
	<b>Dry</b>			<b>Wet</b>		
	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>
Hyundai Accent 4-door	60 mph	137 ft	0.88 g	60 mph	172 ft	0.70 g
Hyundai Accent 2-door L	60 mph	145 ft	0.83 g	60 mph	204 ft	0.59 g
Toyota Tercel 4-door DX	60 mph	156 ft	0.77 g	60 mph	195 ft	0.62 g
Toyota Tercel 2-door base	60 mph	153 ft	0.79 g	60 mph	193 ft	0.62 g
Geo Metro 4-door LSi	60 mph	151 ft	0.80 g	60 mph	172 ft	0.70 g
Geo Metro 2-door LSi	60 mph	152 ft	0.79 g	60 mph	199 ft	0.60 g

<b>Braking performance comparisons of seven 4-wheel drive vehicles on dry roads and on snow. (from Road and Track, April 1989)</b>						
	<b>Dry</b>			<b>Wet</b>		
	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>
BMW 325iX	60 mph	142 ft	0.85 g	20 mph	75 ft	0.18 g
Audi 90 Quattro	60 mph	143 ft	0.84 g	20 mph	99 ft	0.14 g
VW Quantum GL5	60 mph	145 ft	0.83 g	20 mph	59 ft	0.23 g
Toyota Celica All-Trac	60 mph	146 ft	0.82 g	20 mph	80 ft	0.17 g
Subaru Justy 4WD GL	60 mph	151 ft	0.80 g	20 mph	63 ft	0.21 g
Subaru XT6 4WD	60 mph	153 ft	0.79 g	20 mph	49 ft	0.27 g
Pontiac 6000 STE 4WD	60 mph	N/A	N/A	20 mph	56 ft	0.24 g

Appendix B – Spacing and Capacity Evaluations for Different AHS Concepts

<b>Braking performance comparisons on dry roads of passenger vehicles representing extremes (from Road and Track, October 1995)</b>						
	<b>Dry</b>			<b>Wet</b>		
	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>
BMW 325i	60 mph	126 ft	0.95 g	80 mph	212 ft	1.01 g
Chevrolet Corvette LT1	60 mph	123 ft	0.98 g	80 mph	225 ft	0.95 g
Ford Mustang Cobra	60 mph	123 ft	0.98 g	80 mph	214 ft	1.00 g
Toyota Supra Turbo	60 mph	122 ft	0.99 g	80 mph	208 ft	1.03 g
Porsche 911 Turbo	60 mph	116 ft	1.04 g	80 mph	199 ft	1.07 g
BMW 740i	60 mph	144 ft	0.84 g	80 mph	255 ft	0.84 g
Chevrolet Camaro V6	60 mph	162 ft	0.74 g	80 mph	282 ft	0.76 g
Mercury Villager	60 mph	178 ft	0.68 g	80 mph	293 ft	0.73 g
Toyota Corolla DX	60 mph	186 ft	0.65 g	80 mph	319 ft	0.67 g
VW Golf III GL	60 mph	175 ft	0.69 g	80 mph	301 ft	0.71 g

<b>Braking performance comparisons on dry roads of air braked heavy duty vehicles (From NHTSA test data)</b>						
	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>	<b>Initial Velocity</b>	<b>Stopping Distance</b>	<b>Deceler/n (avg. g)</b>
IH School Bus	20 mph	28 ft	0.48 g	60 mph	310 ft	0.34 g
Ford/IH Short School Bus	20 mph	36 ft	0.37 g	60 mph	375 ft	0.32 g
Thomas Transit Bus	20 mph	36 ft	0.37 g	60 mph	292 ft	0.41 g
Ford 4 by 2 Truck	20 mph	36 ft	0.37 g	60 mph	331 ft	0.36 g
GMC 6 by 4 Truck	20 mph	54 ft	0.25 g	60 mph	528 ft	0.23 g
Mack 6 by 4 Truck	20 mph	44 ft	0.30 g	60 mph	363 ft	0.33 g
Peterbilt 4 by 2 Tractor	20 mph	39 ft	0.34 g	60 mph	407 ft	0.30 g
Ford 4 by 2 Tractor	20 mph	30 ft	0.45 g	60 mph	289 ft	0.42 g
White 4 by 2 Tractor	20 mph	42 ft	0.32 g	60 mph	366 ft	0.33 g
IH 6 by 4 Tractor	20 mph	51 ft	0.26 g	60 mph	475 ft	0.25 g
Western Star 6 by 4 tractor	20 mph	46 ft	0.29 g	60 mph	431 ft	0.28 g
Stuart Conv. auto hauler	20 mph	43 ft	0.31 g	60 mph	434 ft	0.28 g
Stuart Stringer auto hauler	20 mph	39 ft	0.34 g	60 mph	354 ft	0.34 g

**APPENDIX B.2 TABLES OF RESULTS**

B.1 Symbols and Notation

PP: Passenger car leader, Passenger car follower

PB: Passenger car leader, Bus follower

PT: Passenger car leader, Truck follower

BP: Bus leader, Passenger car follower

BB: Bus leader, Bus follower

BT: Bus leader, Truck follower

TP: Truck leader, Passenger car follower

TB: Truck leader, Bus follower

TT: Truck leader, Truck follower

$L_P$ : Length of a passenger vehicle, in meters

$L_B$ : Length of a bus, in meters

$L_T$ : Length of a truck with trailer, in meters

$h_{PP}$ : Minimum time headway between Passenger car leader Passenger car follower, in sec.

$h_{PB}$ : Minimum time headway between Passenger car leader, Bus follower, in seconds

$h_{PT}$ : Minimum time headway between Passenger car leader, Truck follower, in seconds

$h_{BP}$ : Minimum time headway between Bus leader, Passenger car follower, in seconds

$h_{BB}$ : Minimum time headway between Bus leader, Bus follower, in seconds

$h_{BT}$ : Minimum time headway between Bus leader, Truck follower, in seconds

$h_{TP}$ : Minimum time headway between Truck leader, Passenger car follower, in seconds

$h_{TB}$ : Minimum time headway between Truck leader, Bus follower, in seconds

$h_{TT}$ : Minimum time headway between Truck leader, Truck follower, in seconds

$V_{io}$ : Leading Vehicle initial Velocity, in miles per hour.

$V_{fo}$ : Following Vehicle initial Velocity, in miles per hour.

$A_{im}$ : The maximum achievable deceleration of the leading vehicle in g

$A_{fm}$ : The maximum achievable deceleration of the following vehicle in g

$J_{lmax}$ : The maximum achievable jerk of the leading vehicle in meters/sec<sup>3</sup>

$J_{fmax}$ : The maximum achievable jerk of the following vehicle in meters/sec<sup>3</sup>

$m_{lmax}$ : The maximum road-tire friction coefficient (dimensionless)

$m_{fmax}$ : The maximum road-tire friction coefficient (dimensionless)

$A_{fauto}$ : The acceleration value under automatic brake control during soft braking, in g

$A_{fac}$ : The initial acceleration value during vehicle following, in g

$J_{fc}$ : The jerk value under automatic brake control during soft braking, in meters/sec<sup>3</sup>

$t_{fa}$ : Detection and brake actuation delay applicable to the following vehicle, in seconds.

$t_{fc}$ : The time at which the following vehicle starts the emergency braking maneuver, in seconds

Appendix B – Spacing and Capacity Evaluations for Different AHS Concepts

**Table B2-1. Autonomous Vehicles, Dry Road Surface**

		<b>PP</b>	<b>PB</b>	<b>PT</b>	<b>BP</b>	<b>BB</b>	<b>BT</b>	<b>TP</b>	<b>TB</b>	<b>TT</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{fo}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.8	0.8	0.8	0.4	0.4	0.4	0.3	0.3	0.3
$A_{tmax}$	g	0.72	0.36	0.27	0.72	0.36	0.27	0.72	0.36	0.27
$J_{lmax}$	$m/s^3$	50	50	50	40	40	40	30	30	30
$J_{tmax}$	$m/s^3$	50	40	30	50	40	30	50	40	30
$m_{lmax}$		1	1	1	1	1	1	1	1	1
$m_{tmax}$		1	1	1	1	1	1	1	1	1
$A_{fauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{fc}$	$m/s^3$	5	5	5	5	5	5	5	5	5
$t_{fa}$	sec	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
$t_{fc}$	sec	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3
min headway	sec	<b>0.66</b>	<b>2.63</b>	<b>3.97</b>	<b>0.08</b>	<b>1.04</b>	<b>2.37</b>	<b>0.06</b>	<b>0.25</b>	<b>1.28</b>
min headway	m	18.71	74.2	111.7	2.37	29.15	66.63	1.71	6.94	36.07

**Table B2-2. Autonomous Vehicles, Wet Road Surface**

		<b>PP</b>	<b>PB</b>	<b>PT</b>	<b>BP</b>	<b>BB</b>	<b>BT</b>	<b>TP</b>	<b>TB</b>	<b>TT</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{fo}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.8	0.8	0.8	0.4	0.4	0.4	0.3	0.3	0.3
$A_{tmax}$	g	0.72	0.36	0.27	0.72	0.36	0.27	0.72	0.36	0.27
$J_{lmax}$	$m/s^3$	50	50	50	40	40	40	30	30	30
$J_{tmax}$	$m/s^3$	50	40	30	50	40	30	50	40	30
$m_{lmax}$		0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
$m_{tmax}$		0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
$A_{fauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{fc}$	$m/s^3$	5	5	5	5	5	5	5	5	5
$t_{fa}$	sec	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
$t_{fc}$	sec	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3
min headway	sec	<b>1.03</b>	<b>4.99</b>	<b>7.65</b>	<b>0.09</b>	<b>1.77</b>	<b>4.43</b>	<b>0.07</b>	<b>0.32</b>	<b>2.26</b>
min headway	m	29.01	140.7	215.6	2.514	49.77	124.7	1.865	9.111	63.57

**Table B2-3. Autonomous Vehicles - Uniform Braking - Dry Road**

		<b>P P</b>	<b>P B</b>	<b>P T</b>	<b>B P</b>	<b>B B</b>	<b>B T</b>	<b>T P</b>	<b>T B</b>	<b>T T</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{to}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.5	0.5	0.5	0.3	0.3	0.3	0.2	0.2	0.2
$A_{tmax}$	g	0.475	0.285	0.19	0.475	0.285	0.19	0.475	0.285	0.19
$J_{lmax}$	m/s <sup>3</sup>	50	50	50	40	40	40	30	30	30
$J_{tmax}$	m/s <sup>3</sup>	50	40	30	50	40	30	50	40	30
$m_{lmax}$		1	1	1	1	1	1	1	1	1
$m_{tmax}$		1	1	1	1	1	1	1	1	1
$A_{fauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{tc}$	m/s <sup>3</sup>	5	5	5	5	5	5	5	5	5
$t_{fa}$	sec	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
$t_{fc}$	sec	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3	0.3
min headway	sec	<b>0.72</b>	<b>2.73</b>	<b>5.25</b>	<b>0.10</b>	<b>1.00</b>	<b>3.52</b>	<b>0.06</b>	<b>0.15</b>	<b>1.36</b>
min headway	m	20.33	76.83	147.7	2.908	28.27	99.15	1.768	4.134	38.19

**Table B2-4. Autonomous Vehicles. Capacity Estimates Under Different Road Conditions Assumptions**

<b>A With Identification of different vehicle classes</b>	<b>Dry Road Surface</b>	<b>Wet Road Surface</b>	<b>Uniform Braking</b>
0% mixing	4116	2860	3850
5% buses	3746	2516	3525
5% trucks	3458	2278	3096
2.5% buses + 2.5% trucks	3596	2391	3297
5% buses + 5% trucks	3193	2054	2882
<b>B. No identification of different vehicle classes</b>	<b>Dry Road Surface</b>	<b>Wet Road Surface</b>	<b>Uniform Braking</b>
0% mixing	4116	2860	3850
5% buses	3631	2432	3416
5% trucks	3356	2207	3007
2.5% buses + 2.5% trucks	3488	2314	3198
5% buses + 5% trucks	3026	1943	2735

Appendix B – Spacing and Capacity Evaluations for Different AHS Concepts

**Table B2-5. Free Agent Vehicles - Infrastructure Supported - Dry Road**

		<b>PP</b>	<b>PB</b>	<b>PT</b>	<b>BP</b>	<b>BB</b>	<b>BT</b>	<b>TP</b>	<b>TB</b>	<b>TT</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{to}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.8	0.8	0.8	0.4	0.4	0.4	0.3	0.3	0.3
$A_{rmax}$	g	0.72	0.36	0.27	0.72	0.36	0.27	0.72	0.36	0.27
$J_{lmax}$	$m/s^3$	50	50	50	40	40	40	30	30	30
$J_{rmax}$	$m/s^3$	50	40	30	50	40	30	50	40	30
$m_{lmax}$		1	1	1	1	1	1	1	1	1
$m_{rmax}$		1	1	1	1	1	1	1	1	1
$A_{fauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{fc}$	$m/s^3$	10	10	10	10	10	10	10	10	10
$t_{fa}$	sec	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$t_{fc}$	sec	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
min headway	sec	<b>0.47</b>	<b>2.44</b>	<b>3.77</b>	<b>0.04</b>	<b>0.84</b>	<b>2.17</b>	<b>0.03</b>	<b>0.12</b>	<b>1.09</b>
min headway	m	13.13	68.7	106.2	1.03	23.64	61.17	0.779	3.34	30.61

**Table B2-6. Free Agent Vehicles - Infrastructure Supported - Wet Road**

		<b>PP</b>	<b>PB</b>	<b>PT</b>	<b>BP</b>	<b>BB</b>	<b>BT</b>	<b>TP</b>	<b>TB</b>	<b>TT</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{to}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.8	0.8	0.8	0.4	0.4	0.4	0.3	0.3	0.3
$A_{rmax}$	g	0.72	0.36	0.27	0.72	0.36	0.27	0.72	0.36	0.27
$J_{lmax}$	$m/s^3$	50	50	50	40	40	40	30	30	30
$J_{rmax}$	$m/s^3$	50	40	30	50	40	30	50	40	30
$m_{lmax}$		0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
$m_{rmax}$		0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
$A_{fauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{fc}$	$m/s^3$	10	10	10	10	10	10	10	10	10
$t_{fa}$	sec	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$t_{fc}$	sec	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
min headway	sec	<b>0.83</b>	<b>4.80</b>	<b>7.46</b>	<b>0.04</b>	<b>1.57</b>	<b>4.23</b>	<b>0.03</b>	<b>0.18</b>	<b>2.06</b>
min headway	m	23.44	135.2	210.1	1.263	44.27	119.2	0.96	5.183	58.11

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**Table B2-7. Free Agent Vehicles - Infrastructure Supported - Uniform Braking - Dry Road**

		<b>P P</b>	<b>P B</b>	<b>P T</b>	<b>B P</b>	<b>B B</b>	<b>B T</b>	<b>T P</b>	<b>T B</b>	<b>T T</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{to}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.5	0.5	0.5	0.3	0.3	0.3	0.2	0.2	0.2
$A_{rmax}$	g	0.475	0.285	0.19	0.475	0.285	0.19	0.475	0.285	0.19
$J_{lmax}$	$m/s^3$	50	50	50	40	40	40	30	30	30
$J_{rmax}$	$m/s^3$	50	40	30	50	40	30	50	40	30
$m_{lmax}$		1	1	1	1	1	1	1	1	1
$m_{rmax}$		1	1	1	1	1	1	1	1	1
$A_{fauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{fc}$	$m/s^3$	10	10	10	10	10	10	10	10	10
$t_{fa}$	sec	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$t_{fc}$	sec	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
min headway	sec	<b>0.53</b>	<b>2.53</b>	<b>5.06</b>	<b>0.05</b>	<b>0.81</b>	<b>3.33</b>	<b>0.03</b>	<b>0.08</b>	<b>1.16</b>
min headway	m	14.79	71.36	142.4	1.347	22.8	93.81	0.863	2.167	32.81

**Table B2-8. Free Agent Vehicles - Infrastructure Supported. Capacity Estimates**

	<b>Dry Road Surface</b>	<b>Wet Road Surface</b>	<b>Uniform Braking</b>
0% mixing	5400	3425	4942
5% buses	4730	2923	4377
5% trucks	4276	2605	3730
2.5% buses + 2.5% trucks	4492	2755	4025
5% buses + 5% trucks	3845	2304	3400

Appendix B – Spacing and Capacity Evaluations for Different AHS Concepts

**Table B2-9. Free Agent Vehicles - Infrastructure Managed - Dry Road**

		<b>P P</b>	<b>P B</b>	<b>P T</b>	<b>B P</b>	<b>B B</b>	<b>B T</b>	<b>T P</b>	<b>T B</b>	<b>T T</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{to}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.8	0.8	0.8	0.4	0.4	0.4	0.3	0.3	0.3
$A_{tmax}$	g	0.72	0.36	0.27	0.72	0.36	0.27	0.72	0.36	0.27
$J_{lmax}$	$m/s^3$	50	50	50	40	40	40	30	30	30
$J_{tmax}$	$m/s^3$	50	40	30	50	40	30	50	40	30
$m_{lmax}$		1	1	1	1	1	1	1	1	1
$m_{tmax}$		1	1	1	1	1	1	1	1	1
$A_{tauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{fc}$	$m/s^3$	20	20	20	20	20	20	20	20	20
$t_{fa}$	sec	0	0	0	0	0	0	0	0	0
$t_{fc}$	sec	0	0	0	0	0	0	0	0	0
min headway	sec	<b>0.36</b>	<b>2.33</b>	<b>3.67</b>	<b>0.01</b>	<b>0.73</b>	<b>2.07</b>	<b>0.01</b>	<b>0.05</b>	<b>0.98</b>
min headway	m	10.25	65.75	103.2	0.411	20.7	58.18	0.327	1.536	27.62

**Table B2-10. Free Agent Vehicles - Infrastructure Managed - Wet Road**

		<b>P P</b>	<b>P B</b>	<b>P T</b>	<b>B P</b>	<b>B B</b>	<b>B T</b>	<b>T P</b>	<b>T B</b>	<b>T T</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{to}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.8	0.8	0.8	0.4	0.4	0.4	0.3	0.3	0.3
$A_{tmax}$	g	0.72	0.36	0.27	0.72	0.36	0.27	0.72	0.36	0.27
$J_{lmax}$	$m/s^3$	50	50	50	40	40	40	30	30	30
$J_{tmax}$	$m/s^3$	50	40	30	50	40	30	50	40	30
$m_{lmax}$		0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
$m_{tmax}$		0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
$A_{tauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{fc}$	$m/s^3$	20	20	20	20	20	20	20	20	20
$t_{fa}$	sec	0	0	0	0	0	0	0	0	0
$t_{fc}$	sec	0	0	0	0	0	0	0	0	0
min headway	sec	<b>0.73</b>	<b>4.69</b>	<b>7.35</b>	<b>0.02</b>	<b>1.46</b>	<b>4.12</b>	<b>0.02</b>	<b>0.11</b>	<b>1.95</b>
min headway	m	20.51	132.1	206.9	0.631	41.18	116.0	0.488	3.033	54.93

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**Table B2-11. Free Agent Vehicles - Infrastructure Managed - Uniform Braking - Dry Road**

		<b>P P</b>	<b>P B</b>	<b>P T</b>	<b>B P</b>	<b>B B</b>	<b>B T</b>	<b>T P</b>	<b>T B</b>	<b>T T</b>
$V_{lo}$	mph	60	60	60	60	60	60	60	60	60
$V_{to}$	mph	63	63	63	63	63	63	63	63	63
$A_{lmax}$	g	0.5	0.5	0.5	0.3	0.3	0.3	0.2	0.2	0.2
$A_{rmax}$	g	0.475	0.285	0.19	0.475	0.285	0.19	0.475	0.285	0.19
$J_{lmax}$	m/s <sup>3</sup>	50	50	50	40	40	40	30	30	30
$J_{rmax}$	m/s <sup>3</sup>	50	40	30	50	40	30	50	40	30
$m_{lmax}$		1	1	1	1	1	1	1	1	1
$m_{rmax}$		1	1	1	1	1	1	1	1	1
$A_{lauto}$	g	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1
$A_{fac}$	g	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
$J_{lc}$	m/s <sup>3</sup>	20	20	20	20	20	20	20	20	20
$t_{fa}$	sec	0	0	0	0	0	0	0	0	0
$t_{fc}$	sec	0	0	0	0	0	0	0	0	0
min headway	sec	<b>0.42</b>	<b>2.43</b>	<b>4.95</b>	<b>0.02</b>	<b>0.70</b>	<b>3.22</b>	<b>0.01</b>	<b>0.04</b>	<b>1.06</b>
min headway	m	11.87	68.38	139.3	0.602	19.82	90.74	0.404	1.119	29.74

**Table B2-12. Free Agent Vehicles - Infrastructure Managed. Capacity Estimates**

	<b>Dry Road Surface</b>	<b>Wet Road Surface</b>	<b>Uniform Braking</b>
0% mixing	6437	3823	5810
5% buses	5472	3197	5018
5% trucks	4873	2820	4184
2.5% buses + 2.5% trucks	5155	2997	4563
5% buses + 5% trucks	4299	2464	3756

Appendix B – Spacing and Capacity Evaluations for Different AHS Concepts

**Table B2-13. Platoons Without Coordinated Braking**

		Dry	Wet	Uniform
$V_{lo}$	mph	60	60	60
$V_{fo}$	mph	61.5	61.5	61.5
$A_{lmax}$	g	0.8	0.8	0.5
$A_{fmax}$	g	0.72	0.72	0.475
$J_{lmax}$	$m/s^3$	50	50	50
$J_{fmax}$	$m/s^3$	50	50	50
$m_{lmax}$		1	0.5	1
$m_{fmax}$		1	0.5	1
$A_{fauto}$	g	0	0	0
$A_{fac}$	g	0	0	0
$J_{fc}$	$m/s^3$	20	20	20
$t_{fa}$	sec	0.1	0.1	0.1
$t_{fc}$	sec	0.1	0.1	0.1
min headway	sec	<b>0.37</b>	<b>0.65</b>	<b>0.38</b>
min headway	m	10.26	17.93	10.48

**Table B2-14. Platoons of Passenger Vehicles Without Coordinated Braking ( $t_{fc}= 0.1$  sec). Capacity Estimates**

	Dry Road Surface	Wet Road Surface	Uniform Braking
<b>A. Autonomous Platoons</b>			
10 car platoons	6090	5652	5955
20 car platoons	6257	5977	6142
<b>B. Free Agent Infrastructure Supported Platoons</b>			
10 car platoons	6312	5843	6166
20 car platoons	6372	6081	6252
<b>C. Free Agent Infrastructure Managed Platoons</b>			
10 car platoons	6434	5947	6283
20 car platoons	6433	6137	6311

**Table B2-15. Platoons with Coordinated Braking - No Delay**

		Dry	Wet	Uniform
$V_{lo}$	mph	60	60	60
$V_{fc}$	mph	61.5	61.5	61.5
$A_{lmax}$	g	0.8	0.8	0.5
$A_{fmax}$	g	0.72	0.72	0.475
$J_{lmax}$	m/s <sup>3</sup>	50	50	50
$J_{fmax}$	m/s <sup>3</sup>	50	50	50
$m_{lmax}$		1	0.5	1
$m_{fmax}$		1	0.5	1
$A_{fauto}$	g	0	0	0
$A_{fac}$	g	0	0	0
$J_{fc}$	m/s <sup>3</sup>	20	20	20
$t_{fa}$	sec	0	0	0
$t_{fc}$	sec	0	0	0
min headway	sec	<b>0.27</b>	<b>0.55</b>	<b>0.28</b>
min headway	m	7.51	15.18	7.73

**Table B2-16. Platoons of Passenger Vehicles with Coordinated Braking ( $t_{fc}= 0$  sec). Capacity Estimates**

	Dry Road Surface	Wet Road Surface	Uniform Braking
<b>A. Autonomous Platoons</b>			
10 car platoons	7217	4531	7028
20 car platoons	7532	4683	7365
<b>B. Free Agent Infrastructure Supported Platoons</b>			
10 car platoons	7531	4652	7323
20 car platoons	7700	4747	7524
<b>C. Free Agent Infrastructure Managed Platoons</b>			
10 car platoons	7704	4718	7489
20 car platoons	7789	4780	7611

**Table B2-17. Platoons with Coordinated Braking.  
(Delay of 0.1 sec from tail to head)**

		Dry	Wet	Uniform
$V_{ic}$	mph	60	60	60
$V_{to}$	mph	61.5	61.5	61.5
$A_{imax}$	g	0.8	0.8	0.5
$A_{tmax}$	g	0.72	0.72	0.475
$J_{imax}$	m/s <sup>3</sup>	50	50	50
$J_{tmax}$	m/s <sup>3</sup>	50	50	50
$m_{imax}$		1	0.5	1
$m_{tmax}$		1	0.5	1
$A_{tauto}$	g	0	0	0
$A_{tac}$	g	0	0	0
$J_{tc}$	m/s <sup>3</sup>	20	20	20
$t_{ta}$	sec	0	0	0
$t_{tc}$	sec	-0.1	-0.1	-0.1
min headway	sec	<b>0.17</b>	<b>0.45</b>	<b>0.18</b>
min headway	m	4.76	12.431	4.98

**Table B2-18. Platoons of Passenger Vehicles with Coordinated Braking ( $t_{fc} = -0.1$  sec). Capacity Estimates**

	Dry Road Surface	Wet Road Surface	Uniform Braking
<b>A. Autonomous Platoons</b>			
10 car platoons	7060	4468	6889
20 car platoons	7442	4646	7291
<b>B. Free Agent Infrastructure Supported Platoons</b>			
10 car platoons	7359	4586	7171
20 car platoons	7604	4709	7445
<b>C. Free Agent Infrastructure Managed Platoons</b>			
10 car platoons	7525	4649	7330
20 car platoons	7692	4743	7530

**Table B2-19. Infrastructure Managed Slotting. Capacity Estimates**

	Dry Road Surface	Wet Road Surface	Uniform Braking
0% mixing	4047	2826	3773

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**Table B2-20. Capacity Comparisons**

Capacity without platooning	0% mixing of vehicles			5% mixing of buses			5% mixing of trucks		
	Dry	Wet	Uni-form	Dry	Wet	Uni-form	Dry	Wet	Uni-form
Autonomous Vehicles with class identification	4116	2860	3850	3746	2516	3525	3458	2278	3096
Autonomous Vehicles without class identification	4116	2860	3850	3631	2432	3416	3356	2207	3007
Free Agents - Infrastructure Supported with class identification	5400	3425	4942	4730	2923	4377	4276	2605	3730
Free Agents - Infrastructure Managed with class identification	6437	3823	5810	5472	3197	5018	4873	2820	4184
Infrastructure Managed Slotting	4047	2826	3773						
				<b>2.5% buses + 2.5% trucks</b>			<b>5% buses + 5% trucks</b>		
				<b>Dry</b>	<b>Wet</b>	<b>Uni-form</b>	<b>Dry</b>	<b>Wet</b>	<b>Uni-form</b>
Autonomous Vehicles with class identification				3596	2391	3297	3193	2054	2882
Autonomous Vehicles without class identification				3488	2314	3198	3026	1943	2735
Free Agents - Infrastructure Supported with class identification				4492	2755	4025	3845	2304	3400
Free Agents - Infrastructure Managed with class identification				5155	2997	4563	4299	2464	3756
Capacity with platooning	10 car platoons			20 car platoons					
	Dry	Wet	Uni-form	Dry	Wet	Uni-form			
Autonomous platoons without coordinated braking	6090	5652	5955	6257	5977	6142			
Infrastructure supported platoons without coordinated braking	6312	5843	6166	6372	6081	6252			
Infrastructure managed platoons without coordinated braking	6434	5947	6283	6433	6137	6311			
Autonomous platoons with coordinated braking	7217	4531	7028	7532	4683	7365			
Infrastructure supported platoons with coordinated braking	7531	4652	7323	7700	4747	7524			
Infrastructure managed platoons with coordinated braking	7704	4718	7489	7789	4780	7611			
Autonomous platoons with delayed braking	7060	4468	6889	7442	4646	7291			
Infrastructure supported platoons with delayed braking	7359	4586	7171	7604	4709	7445			
Infrastructure managed platoons with delayed braking	7525	4649	7330	7692	4743	7530			